



GEOTECHNICAL INVESTIGATION REPORT

GEOTECHNICAL INVESTIGATION
Proposed Industrial Warehouse
NE Corner of Cardova Road and Dachshund Avenue
Apple Valley, CA

Prepared for Redwood West
Submitted by Merrell Johnson Geotechnical, Inc.
December 1, 2023

MERRELL JOHNSON

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December 1, 2023

Butterfly Equity Partners and /or
RW Apple Valley II and /or
Assignee, and/or Linkup, LLC
220 Newport Center Drive, Ste 11-557
Newport Beach, CA 92660

Re: Geotechnical Investigation Report | Proposed Industrial Warehouse | NE Corner of Cardova Road and Dachshund Avenue, Apple Valley, CA 92307 | M.J.G. Project No. 3813.006.500

Ladies and Gentlemen:

This letter transmits Merrell Johnson Geotechnical's Geotechnical Investigation Report for the subject industrial warehouse building. The investigation was planned and performed based on the proposed project development illustrated on the Conceptual Grading Plan prepared by Merrell Johnson, dated September 18, 2023.

We trust that the enclosed information will be useful for the design and construction phases of this project. If you have any questions, please do not hesitate to contact our firm.

Sincerely,

Merrell Johnson Companies



Brad S. Merrell, P.E., President
Merrell Johnson Geotechnical, Inc.
R.C.E. 49423



Jeff S. Burns, Project Manager
Merrell Johnson Geotechnical, Inc.

MERRELL JOHNSON

CONTENTS

Introduction.....	2
Project Description.....	2
Scope of Services.....	2
Field Exploration and Laboratory Testing.....	2
Field Exploration.....	2
Laboratory Testing.....	3
Site and Subsurface Conditions.....	3
Site Conditions.....	3
Subsurface Conditions.....	3
Expansion Potential.....	3
Geologic Setting.....	4
CBC Ground Motion Analysis.....	4
Groundwater.....	5
Faulting.....	6
Conclusions and Recommendations.....	7
Compacted Fill Placement.....	7
Imported Soils.....	7
Lateral Loading.....	8
Building Floor Slabs.....	8
Surface Drainage.....	9
Concrete and Flexible Asphalt Concrete Pavement Subgrade Preparation.....	9
Flexible Pavement Structural Sections.....	10
Onsite Flexible Pavement.....	10
Off-Site Flexible Pavement for Cordova and Dachshund Avenue.....	11
Portland Cement Concrete Pavements and Flatwork.....	12
Utility Excavations.....	12
Corrosivity.....	13
Limitations.....	13

APPENDICES

Appendix A	Figures
<i>Figure 1</i>	Site Vicinity Map
<i>Figure 2</i>	Plot Plan / Satellite Site Image
<i>Figure 3</i>	Architectural Site Plan
<i>Figure 4</i>	Conceptual Grading Plan & Test Boring Plot Plan
Appendix B	Exploratory Logs
Appendix C	Laboratory Testing

INTRODUCTION

Project Description

This report presents the results of the geotechnical investigation Merrell Johnson Geotechnical (MJG) performed for the new warehouse planned for the 68.2-acre parcel located on the northeast corner of Cardova and Dachshund Avenue, Apple Valley, CA. The location of the proposed development is shown on the Site Vicinity Map, Google Earth Site Image, Site Plan, and Conceptual Grading Plan, included with this report as Appendix A, Figures 1-4.

The proposed industrial warehouse will occupy a plan area of 1,346,270 square feet (SF) and includes a 26,880 SF mezzanine. Proposed site improvements will include truck loading docks, truck trailer parking, automobile parking, driveways, retention basin, utilities, and other ancillary improvements. Maximum cut and fill depths required to grade the site are anticipated to be about 15 feet and 10 feet, respectively, plus building pad overexcavation.

Proposed off-site improvements will include installing utilities and paving Dachshund Avenue from Quarry Road south to Johnson Road and Cardova Road westward from the project's southeast corner to Dale Evans Parkway.

Scope of Services

The scope of work for this project consisted of field exploration, laboratory testing, engineering analyses, and preparation of this report. The results of the field exploration and laboratory test programs were analyzed to develop conclusions and recommendations regarding:

- Subsurface conditions underlying areas to be developed
- Site preparation and grading
- Excavation conditions
- Foundation support for the new structure along with soils engineering criteria for foundation design
- Support for slab-on-grade floors
- Concrete and flexible pavement structural sections for parking and driveway/fire lanes
- Flexible pavement structural sections for Dachshund Avenue and Cardova Road off-site street improvements

FIELD EXPLORATION AND LABORATORY TESTING

Field Exploration

Subsurface conditions were explored by drilling five (5) test borings within the warehouse building limits, one (1) to a depth of 50 feet and four (4) to a depth of 25 feet. The 50-foot-deep test boring was also used to evaluate the soil liquefaction potential.

Outside the limits of the proposed warehouse, two 10-foot-deep test borings were drilled within the proposed retention basin, six 5-foot-deep test borings within the onsite loading/parking areas, and four 5-foot-deep testing borings within the off-site Cardova and Dachshund Avenue improvement areas.

The locations of the test borings are shown on Conceptual Grading Plan, Figure 3 in Appendix A. The borings were logged by an MJG's representative, who also collected samples of the materials encountered for examination and laboratory testing.

Bulk samples were collected from drill cuttings. Relatively undisturbed samples were obtained by driving a 2.5-inch inside diameter modified California sampler with a 140-pound hammer falling 30 inches. Blow counts required to drive the sampler each 6 inches of an 18-inch (or less) drive are noted on the boring logs as “N” value.

Standard Penetration Tests (SPTs) were performed at selected depths by driving a 1.4-inch inside diameter sampler 18 inches with a 140-pound hammer falling 30 inches. The blow counts required to drive the sampler each 6 inches of the drive are noted on the boring logs as “N” value. Disturbed samples were collected from the SPT sampler at the time of driving.

The logs of the test borings are in Appendix B. Soils are described according to the Unified Soil Classification System explained in Appendix B.

Laboratory Testing

The laboratory program included the following tests:

- ASTM D 422 - Grain Size Analysis
- ASTM D 1557 - Maximum Density
- ASTM D 2419 – Sand Equivalent
- ASTM D 2435 / CT-219 - Consolidation of Soils
- ASTM D 2844 / CT 301 - Resistance to R-value
- ASTM D 2937 - In-place Moisture Content and Dry Density
- ASTM D 3080 - Direct Shear
- ASTM D 4829 - Expansion Index of Soil
- G51/CT643, CT417, CT422 - Corrosion Potential

The results of the laboratory tests are summarized in Appendix C.

SITE AND SUBSURFACE CONDITIONS

Site Conditions

The 68.2-acre site is located on the northeast corner of Cordova and Dachshund Avenue. The property is undeveloped and covered with sparse desert vegetation. The ground surface slopes (descends) gradually towards the southwest at an inclination of about 1.5 percent. Cordova and Dachshund Avenue are unpaved. Quarry Road bounds the north side of the site. Quarry Road is a paved private road with no legal paved access to the project site.

Subsurface Conditions

The site is blanketed by medium dense silty sand (SM) to a depth of about 3 feet where it becomes dense. At a depth of about 5 feet, dense poorly graded sand with silt with some gravel and cobbles (SP-SM) was encountered and extended to the depth explored (50 feet). A boulder was encountered in Boring No. 1 at a depth of 30 feet.

Expansion Potential

Results of an expansion index test (ASTM D4829) performed on a near-surface soil sample from Boring No. 3 within the proposed warehouse limits exhibited an expansion index of 2, which corresponds to a very low expansion potential.

Geologic Setting

The subject site is located within a natural geomorphic province in southern California known as the Mojave Desert. This province consists of a broad interior region of isolated mountain ranges separated by expanses of desert plains and is characterized by the numerous interior enclosed drainages and playas. The Mojave Desert is in large, bounded structurally on the southwest by the San Andreas Fault and on the northwest by the Garlock Faults, and is ill-defined along the east where the structural patterns resemble the Basin and Range Province to the north and east. This province exhibits interior drainage, including the Mojave River, which has its source in the San Bernardino Mountains and would extend into Death Valley if there was enough water.

The geologic units of this region generally consist of three main divisions being: 1) Crystalline rocks of pre-Tertiary age; 2) sediments and volcanic rocks of Tertiary age; and 3) sediments and basalt flows of Quaternary age. Regionally, the site is located along a large alluvial plain, locally underlain by Quaternary age alluvium and older that has been derived predominantly as outwash from the adjacent highlands to the north and east along Sidewinder Valley. These sediments are believed to be less than 500± feet locally (Subsurface Surveys, 1990).

Locally as mapped by Hernandez and Tan (2007), the subject site is shown to be underlain by late Pleistocene age older alluvium. These deposits are generally described as being comprised of a fine- to medium-grained sand and fine to medium gravel of inactive fans.

CBC Ground Motion Analysis

Included for this study was an assessment of the seismic ground motion parameters of the subject site with respect to the most recently adopted 2022 California Building Code (CBC) and ASCE/SEI Standard 7-22 (ASCE, 2022) as partially summarized and tabulated below.

Geographically, the proposed construction area is centrally located at Latitude 34.610365 and Longitude -117.196553.

Site Classification (CBC 1613.3.2) – Based on the presence of mapped Quaternary age alluvial deposits underlying the site and the absence of site-specific shear-wave data, the design Site Class is *estimated* to be **“D.”** This Class is defined as having the upper 100 feet (30 meters) of the subsurface being underlain by “Stiff Soil” with average shear-wave velocities of 600 to 1,200 feet/second (180 to 360 meters/second). In accordance with the CBC, the proposed warehouse is considered a Risk Category II structure.

Factor or Coefficient	Value
S_s	1.16g
S_1	0.39g
S_{Ds}	1.0g
S_{D1}	0.67
S_{Ms}	1.5g
S_{M1}	1.0
T_L	12 Seconds
PGA_M	0.55g
V_{S30}	260
Site Class	D

Groundwater

The study area lies within the Upper Mojave River Groundwater Basin of southern California. The Mojave River Basin is part of the Mojave Desert region and is bordered by the San Bernardino and San Gabriel Mountains to the south and extends to Afton Canyon to the northeast, with Lucerne Valley and Antelope Valleys bordering the east and west, respectively. The Mojave River, which is located to the west, is the principal source of water recharge to the basin, which originates from the junctions of Deep Creek and West Fork Mojave River at the northern foot of the San Bernardino Mountains. Other sources of recharge include other lesser river tributaries from the San Bernardino and San Gabriel Mountains, the adjacent highlands to the north and east, as well as deep percolation from rainwater and other artificial means.

The water-bearing deposits are principally unconsolidated and partially consolidated continental sedimentary deposits that form two aquifers (Stamos and Predmore, 1995), the upper one being shallow alluvium (200± feet thick, within 1± mile of the Mojave River), with the regional aquifer underlying most of the basin at depth. The regional aquifer is comprised of unconsolidated older alluvium and fan deposits of Pleistocene to Tertiary age, and partly consolidated to consolidated sediments of Tertiary age. These deposits are as much as 1,000 feet thick in some places and their permeability generally decreases with depth.

Based on groundwater data provided by the California Department of Water Resources (2023), the closest measured well is approximately 1,300± feet to the west (State Well No. 06N03W16C001S), which had a water level of 39 feet in 1957 (only measurement). Other wells in the nearby vicinity show groundwater depth around 87 feet in depth (State Well No. 06N03W16D001S) located approximately 2,400± feet to the west, and 52 feet in depth (State Well No. 06N03W09N001S) located approximately 2,400± feet to the northwest.

Faulting

There are at least thirty-five major "potentially active/active" (late Quaternary) faults that are within a 100-kilometer (62 mile) radius of the site as shown on Figure 1 below (site shown as small dot in middle).

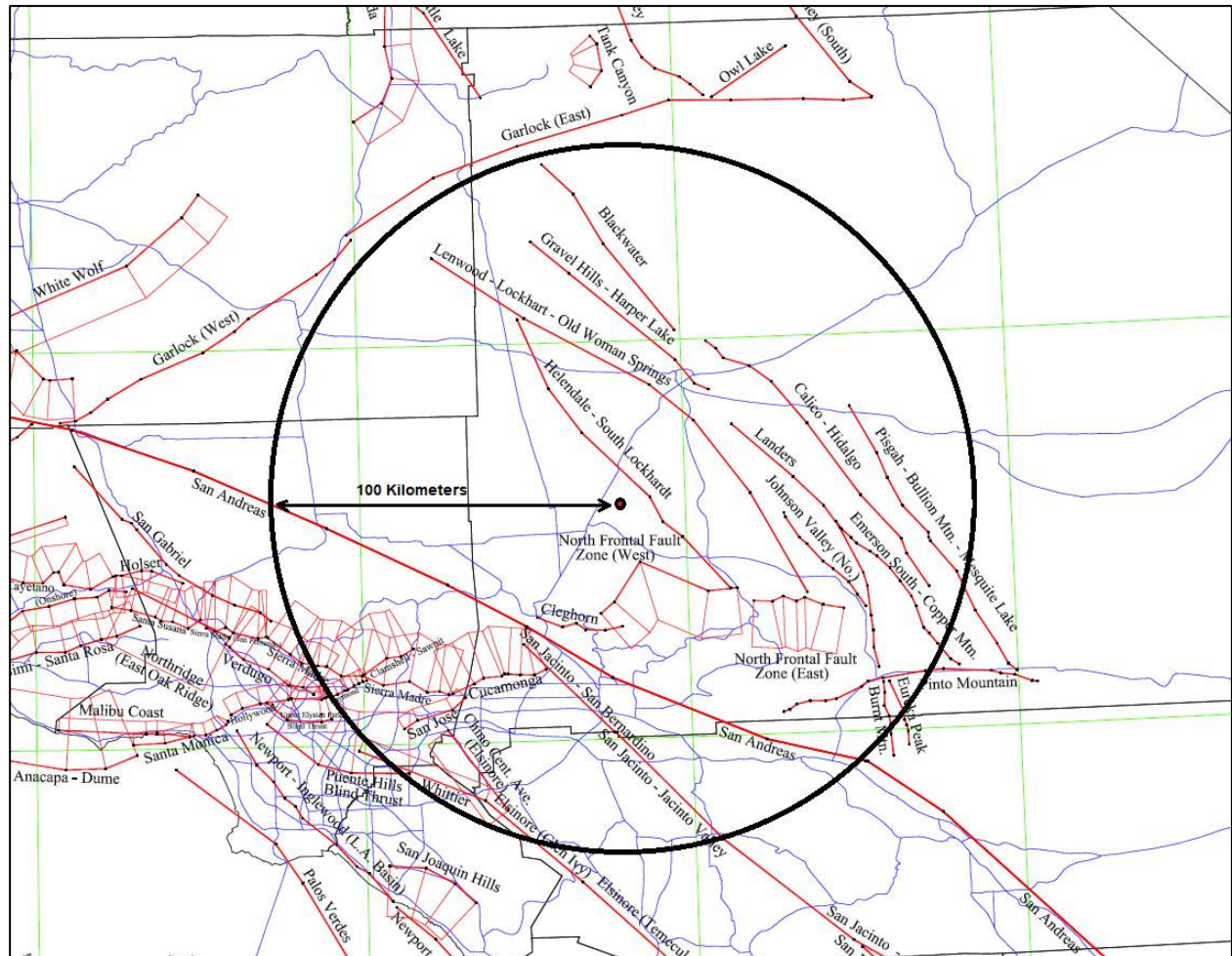


FIGURE 1: Regional Fault Map showing 100 km radius (from CGS 2002 California Fault Model)

Of these, there are no active faults known to traverse the site based on published literature. In addition, the subject site is not located within a State of California "Alquist-Priolo Earthquake Fault Zone" for surface fault rupture hazards (CGS, 2018).

The nearest mapped "active" fault zoned by the State of California is for the Helendale-South Lockhart Fault, located approximately $2\frac{1}{2}\pm$ miles to the northeast. The Helendale-South Lockhart Fault is a right-lateral, strike-slip fault, being approximately 97 kilometers in length, with an associated slip-rate of 0.6 ± 0.4 mm/year (C.D.M.G., 1996 and Cao, et al., 2003).

CONCLUSIONS AND RECOMMENDATIONS

The existing surface soils are medium dense to a depth of about 3 feet. Below 3 feet, the native soils are generally dense and are considered adequate for support of the new facilities.

The site is not within an Alquist Priolo Earthquake Fault Zone. The soils are medium dense to dense, and groundwater is deep below this site. The liquefaction potential consequently is very low. The potential for dynamically induced settlement of the granular soils is also very low. In addition, the soils have a very low potential for expansion due to changes in moisture content.

The potential for encountering groundwater within the anticipated relatively shallow excavations is minimal. There is a potential for minor amounts of water to enter open excavations because of direct rainfall and runoff.

Earthwork

At the time of MJG's investigation, the site was covered with sparse desert vegetation. Any debris, vegetation, and other deleterious materials should be stripped and removed from the site prior to grading work. Organic materials should be disposed of off-site in accordance with the owner's instructions. Roots should be removed to a depth of 6 inches below foundation and pavement subgrade elevations.

Areas to receive fill should be scarified to a depth of 12 inches, brought to within 2 percentage points above or below optimum moisture content, and compacted to a minimum of 95% relative compaction based on the ASTM D1557 laboratory test method. All references to optimum moisture content and relative compaction in this report are based on this test method.

Compacted Fill Placement

Fill should be placed in 8-inch-thick loose lifts, moisture conditioned to within 2 percentage points above or below optimum moisture content and compacted to a minimum of 95% relative compaction.

Imported Soils

Imported soils, if needed, should consist of predominantly granular material with an expansion index less than 20 when tested in accordance with ASTM D4829, and should have a minimum R-value of 40. Imported material should be inspected and approved by an MJG's representative prior to being brought to the site.

Shallow Foundation and Building Slab-On-Grade Support

The existing soils below and within five (5) feet of the proposed warehouse building should be over-excavated to a depth of at least three feet below the existing ground surface or 12 inches below the proposed footing base grade, whichever depth is greater. The bottom of the over-excavation should be scarified to a depth of at least 6 inches, moistened to within 2 percent of the optimum moisture content, and compacted to a relative compaction of at least 95 percent (ASTM D 1557).

Fill should be placed in 8-inch-thick loose lifts, moisture conditioned to within 2 percentage points above or below optimum moisture content and compacted to a minimum of 95% relative compaction.

The planned structures can be supported on shallow spread footings with bottom levels in the compacted fill at a minimum depth of 18 inches below the lowest adjacent finished grade.

A minimum width of 18 inches is recommended for continuous footings. Isolated footings should be at least 24 inches wide. Footings can be designed for an allowable bearing pressure of 2500 pounds per square foot (psf) for dead plus long-term live loads. This value can be increased by $\frac{1}{3}$ when considering the total of all loads, including wind or seismic forces.

Total post-construction settlement is estimated to be approximately $\frac{3}{4}$ inch. Post-construction differential settlements are anticipated to be $\frac{1}{2}$ inch or less between isolated footings, and between the middle and end of a continuous footing.

Continuous (strip) foundations should be reinforced with a minimum of #5 deformed reinforcing bars at the top and bottom of the footings.

Spread footing reinforcement should be designed by the structural engineer for punching shear and bending. As a minimum, the spread footings should be reinforced with a #5 deformed reinforcing bars, spaced 18 inches on center each way and placed 3 inches above the bottom of the spread footing.

All grade beam reinforcement should be designed and specified by the building's designer/structural engineer.

Foundations should be reinforced as necessary to reduce the potential for distress caused by differential foundation movement. The use of joints at openings or other discontinuities in masonry walls is recommended.

Footing excavations should be observed by an MJG's representative to check bearing materials and cleaning.

Lateral Loading

Resistance to lateral loads will be provided by passive earth pressure against the faces of footings and other structural elements below grade, and by friction along the bases of footings and slabs. Passive earth pressure can be taken as 350 pounds per square foot (psf) per foot of depth. Base friction can be taken as 0.35 times the actual dead load. Base friction and passive earth pressure can be combined without reduction. Retaining structures free to rotate at the top should be designed for an active equivalent fluid pressure of 35 psf per foot of height, plus any additional building or equipment surcharge. MJG should be notified if retaining walls greater than 10 feet in height, restrained walls, or tieback walls are planned so that geotechnical recommendations specific to wall conditions can be developed.

Building Floor Slabs

During grading operations, the building pad soils should be compacted to a relative compaction of at least 95 percent (ASTM D 1557). Prior to placing the slab-on-grade concrete, the final pad surfaces should be proof-rolled to provide a smooth, dense surface upon which to place the concrete.

A 15-mil vapor retarder membrane, conforming to ASTM E 1745 and installed per ASTM E 1643, should be placed beneath concrete slabs-on-grade covered with moisture sensitive or impervious floor coverings, or where the slab will support equipment or materials sensitive to moisture. When conditions warrant the use of a vapor retarder, the slab designer should refer to

ACI 302 and/or ACI 360 for procedures and cautions regarding the use and placement of a vapor retarder.

Reinforcing for slabs-on-grade should be designed by the project structural engineer based on anticipated storage and forklift loads. A modulus of subgrade reaction of 150 pounds per cubic inch (pci) can be used. Reinforcing should extend down into the footings. Concrete construction (i.e. jointing, etc.) should be in conformance with the American Concrete Institute Manual of Concrete Practice Design and Construction Standards.

Minimum reinforcing for 4-inch-thick slabs-on-grade should consist of 6" x 6" W1.4 /1.4 welded wire fabric supported mid height in the slab by concrete blocks or dobies. Positioning the wire fabric by lifting after concrete placement should not be allowed. Minimum reinforcing for 6-inch-thick slabs-on-grade should consist of at least #4 deformed reinforcing bars at 12 inches on center each way placed at mid-height in the slab.

Where the project's structural engineer's reinforcement recommendations exceed MJG's above minimum slab reinforcement recommendations, the structural engineer's recommendations should be followed.

Saw-cut control joints should be placed in the slab to help control the location and extent of cracking. For additional recommendations refer to the ACI Design Manual. Joints or cracks should be sealed with a waterproof, non-extruding compressible compound specifically recommended for heavy duty concrete pavement and wet environments.

Where floor slabs are tied to perimeter walls or turn-down slabs to meet structural or other construction objectives, our experience indicates differential movement between the walls and slabs will likely be observed in adjacent slab expansion joints or floor slab cracks beyond the length of the structural dowels. The structural engineer should account for potential differential settlement through use of sufficient control joints, appropriate reinforcing, or other means.

Surface Drainage

It is important that water be kept a minimum of 5 feet from structures and slabs. No ponding adjacent to buildings and structures should be allowed. Final surfaces should have a positive 2 percent minimum slope away from structures.

Retaining walls should be designed to resist hydrostatic pressures or be provided with a back-drain, weep holes or other drainage facilities. If a basement or underground structure is constructed, a subsurface drainage system is recommended.

Concrete and Flexible Asphalt Concrete Pavement Subgrade Preparation

Soils should be scarified to a depth of at least 12 inches, moistened to within 2 percent of the optimum moisture content and compacted to a relative compaction of at least 95 percent (ASTM D1557). Any additional fill required to prepare the finish subgrade should be placed in maximum 8-inch-thick lifts, each lift moistened to within two (2) percent of the optimum moisture content and compacted to a relative compaction of at least 95 percent.

Flexible Pavement Structural Sections

New flexible pavement structural sections were determined following California Department of Transportation (Caltrans) procedures.

ONSITE FLEXIBLE PAVEMENT

Onsite flexible pavement for automobiles and light trucks are planned for the north and south sides of the project site. MJG's review of the Conceptual Site Plan indicates that most of the heavy truck driveways, trailer parking areas, and loading docks will be located on the east and west sides of the warehouse and paved with rigid portland cement concrete pavement. The automobile and truck trailer parking areas will be connected by onsite access driveways.

Six test borings were drilled around the perimeter of the proposed warehouse. Sieve analyses (ASTM D422, D1140 & D2487) and sand equivalent tests (ASTM D 2844) were performed from soil samples collected in each of these borings. The soil samples containing the largest percentage of fines (soil fraction passing the #200 sieve) and the lowest sand equivalent value were selected and tested for R-value (ASTM D 2844).

The most critical soil samples were collected in Boring Nos. 10 and 12 located on the east and west sides of the warehouse, respectively. R-value tests were performed from the soil samples collected in these two borings. The sieve analysis, sand equivalent, and R-value test data are summarized in Table 2 below.

Boring No.	% Passing the 200 Sieve (ASTM D422, D1140, & D2487)	Sand Equivalent ASTM D 2844	R-value ASTM D2844
8	10.1	71	--
9	12.4	36	--
10	13.9	24	25
11	18.4	24	--
12	17.4	17	17
13	14.9	37	--

Based on the above soil test data, the most critical R-value of 17 was used to calculate the recommended onsite flexible pavement sections for the project. In addition, the Traffic Indexes for the flexible pavement areas was selected as follows:

- Automobiles and light truck traffic: TI = 5
- Heavy truck traffic: TI = 8

Recommended onsite flexible pavement structural sections are listed in Table 3 below.

<i>Pavement Area</i>	<i>Traffic Index</i>	<i>Asphalt Concrete (inches)</i>	<i>Base Course (inches)</i>
Autos & Light Truck Traffic	5	3	8
Heavy Truck Traffic	8	6	12

OFF-SITE FLEXIBLE PAVEMENT FOR CARDOVA AND DACHSHUND AVENUE

Off-site flexible pavement is proposed for Cardova Road and Dachshund Avenue adjacent the subject warehouse site. Additionally, the paving of Cardova and Dachshund Avenue will be extended from the southwest corner of the warehouse site westward to Dale Evans Parkway and southward to Johnson Road, respectively.

Two test borings were excavated along the alignment of Cardova Road and two along Dachshund Avenue (4 total). Samples from the four test borings were collected and tested for sieve analysis (ASTM D422, D1140 & D2487). The samples containing the largest percentage of fines from each street (soil fraction passing the #200 sieve) were selected and tested for R-value (ASTM D 2844).

The most critical soil samples were collected in Boring Nos. 14 and 15 located on Cardova Road and Dachshund Avenue, respectively. R-value tests were performed from the soil samples collected in these two borings. The sieve analysis and R-value test data are summarized in Table 4 below.

<i>Boring No.</i>	<i>Roadway</i>	<i>% Passing the 200 Sieve (ASTM D422, D1140, & D2487)</i>	<i>R-value ASTM D2844</i>
14	Dachshund Avenue	23.4	58
15	Cardova Road	27.4	52
16	Cardova Road	15.4	--
17	Dachshund Avenue	20.1	--

Based on the above soil test data, the most critical R-values of 52 and 58 were used to calculate the recommended off-site flexible pavement sections for Cardova and Dachshund Avenue, respectfully. The Traffic Index used to calculate the roadway pavement sections was:

- Automobiles and Heavy truck traffic: TI = 9

Recommended off-stie flexible pavement structural sections for Cardova and Dachshund Avenue are presented in Table 5 below.

Roadway	Traffic Index	Asphalt Concrete (inches)	Base Course (inches)
Cardova Road	9	5	6
Dachshund Avenue	9	5.5	4

Alternative pavement sections can also be considered. Merrell Johnson should review the design pavement sections if traffic loads will be different from those currently anticipated.

PORTLAND CEMENT CONCRETE PAVEMENTS AND FLATWORK

The subgrade surface beneath rigid (portland cement concrete) pavements should be proof-rolled with a smooth-wheel roller to form a dense, uniform surface. Any pumping or yielding areas should be excavated and replaced with compacted fill.

Rigid pavements to support automobile and light truck traffic should be a minimum of 6 inches thick and reinforced with a minimum of #4 deformed reinforcing bars spaced 12 inches on center each way. Joints should be provided at intervals of no more than 12 feet. Smooth dowels should be provided across pavement joints.

Rigid pavement to support heavy truck traffic should be a minimum of 8 inches thick and reinforced with #4 deformed reinforcing bars spaced 12 inches of center each way. Joints should be provided at intervals of no more than 12 feet. Smooth dowels should be provided across pavement joints.

Pedestrian walkways and other lightly loaded concrete flatwork areas should be proof rolled as described above. The flatwork in these areas should have a minimum thickness of 4 inches and be provided with doweled joints at no more than 12-foot intervals. Minimum reinforcement should consist of 6 x 6 W1.4/1.4 welded wire fabric supported mid height in the slab by concrete blocks or dobies. Positioning the wire fabric by lifting after concrete placement should not be allowed.

UTILITY EXCAVATIONS

Excavations for this project will require sloping sidewalls or shoring. Excavations should be made in accordance with California Administrative Code, Title 8, Industrial Relations, Chapter 4, Division of Industrial Safety, Subchapter 4, Construction Safety Orders, Article 6. Temporary excavations should be shored or sloped in accordance with Cal OSHA requirements. On-site soils can be considered Type C for purposes of excavation design.

In general, temporary excavations in on-site soils should be sloped no steeper than 1.5 horizontal to 1 vertical for excavations up to 20 feet in depth. Compound excavations with vertical sides in lower portions should be properly shielded to a minimum height of 18 inches above the top of the vertical side, with the upper portion having a maximum slope of 1.5 horizontal to 1 vertical. A Registered Professional Engineer should design slopes or benching for excavations greater than 20 feet in depth.

Temporary excavation slopes should be inspected twice daily by the contractor's competent person before personnel are allowed to enter the excavation. If sloughing, raveling or other evidence for slope instability is noted, corrective measures should be implemented.

Temporary shoring will be required for those excavations where temporary cut slopes as described above are not feasible. Cantilever shoring, and shoring with 1 level of bracing, can be designed to resist an equivalent fluid pressure of 30 psf per foot of depth. For shoring with multiple levels of bracing, a uniform lateral pressure equal to $25H$ in psf, where H is the height of shoring in feet, should be used. The recommended soil pressure applies to level soil conditions behind the shoring. Where a combination of sloped embankment and shoring is used, the soil pressure will be greater and should be evaluated for actual conditions.

In addition to the above recommended lateral earth pressures, a minimum uniform lateral pressure of 125 psf should be incorporated in the design of the upper 10 feet of shoring when normal traffic is permitted within 10 feet of the shoring. The design of temporary shoring should also include the surcharge loads from delivery and construction equipment, as appropriate.

CORROSIVITY

Laboratory test results indicate that resistivity of the soils sampled exhibit resistivities ranging from 260 to 1,500 ohms-cm, which indicates the soils have a corrosion potential with respect to reinforced concrete and ferrous metals. For this reason, Type II modified, or Type V cement is recommended for use in concrete in contact with the ground. Foundations should be designed with continuous reinforcing steel top and bottom. Reinforcing steel should maintain minimum clearances specified by applicable codes and good construction practice. Appropriate corrosion protection, including consultation with a qualified corrosion engineer, should be implemented anywhere ferrous metal is in contact with the soils.

LIMITATIONS

The recommendations in this report are based on results of the field exploration and laboratory test programs, combined with interpolation and extrapolation of subsurface conditions between and beyond boring locations. The nature and extent of variations in these conditions may not become evident until construction. If variations are encountered during construction, MJ should be notified so these variations can be reviewed and the recommendations in this report modified if necessary. If changes in the nature, design or location of the structures are planned, these changes should be reviewed by MJ so that modifications to the recommendations in this report can be made if needed.

Our professional services have been performed using the degree of care and skill ordinarily exercised under similar circumstances by reputable engineering consultants practicing in this or similar localities. No other warranty, express or implied, is made as to the professional advice or data included in this report. This report has not been prepared for use by other parties and may not contain sufficient information for purposes of other parties or other uses.

APPENDIX A

Figure 1 – Site Vicinity Map

Figure 2 – Plot Plan / Satellite Site Image

Figure 3 – Architectural Site Plan

Figure 4 – Conceptual Grading Plan

Industrial Warehouse Site

Legend
Apple Valley
Black Mountain Quarry Plant



Black Mountain Quarry Plant

Industrial Warehouse Site

Bell Mountain

Victorville

DESERT KNOLLS

Apple Valley
Apple Valley

ande

ive Heights

E

Google Earth

Spring Valley Lake

2 mi

Industrial Warehouse Site

Legend

- Apple Valley
- Black Mountain Quarry Plant

Mountain

Quarry Rd

Quarry Rd

Date Evans Pkwy

Date Evans Pkwy

Industrial Warehouse Site

Cardova Rd

Cardova Rd

Comanche Rd

Dachshund Ave

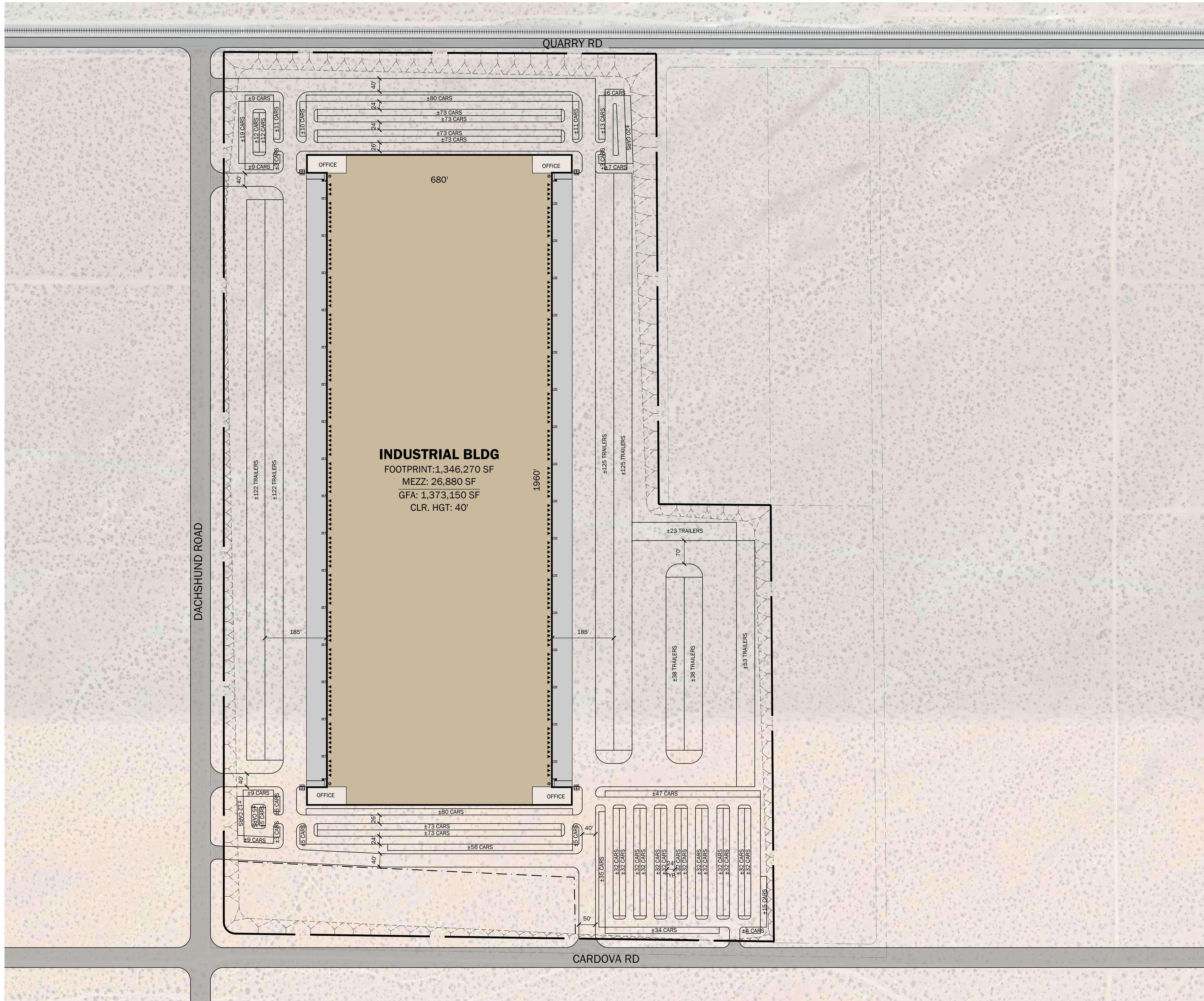
Navajo Rd

Doberman St

Shepherd Rd

Google Earth



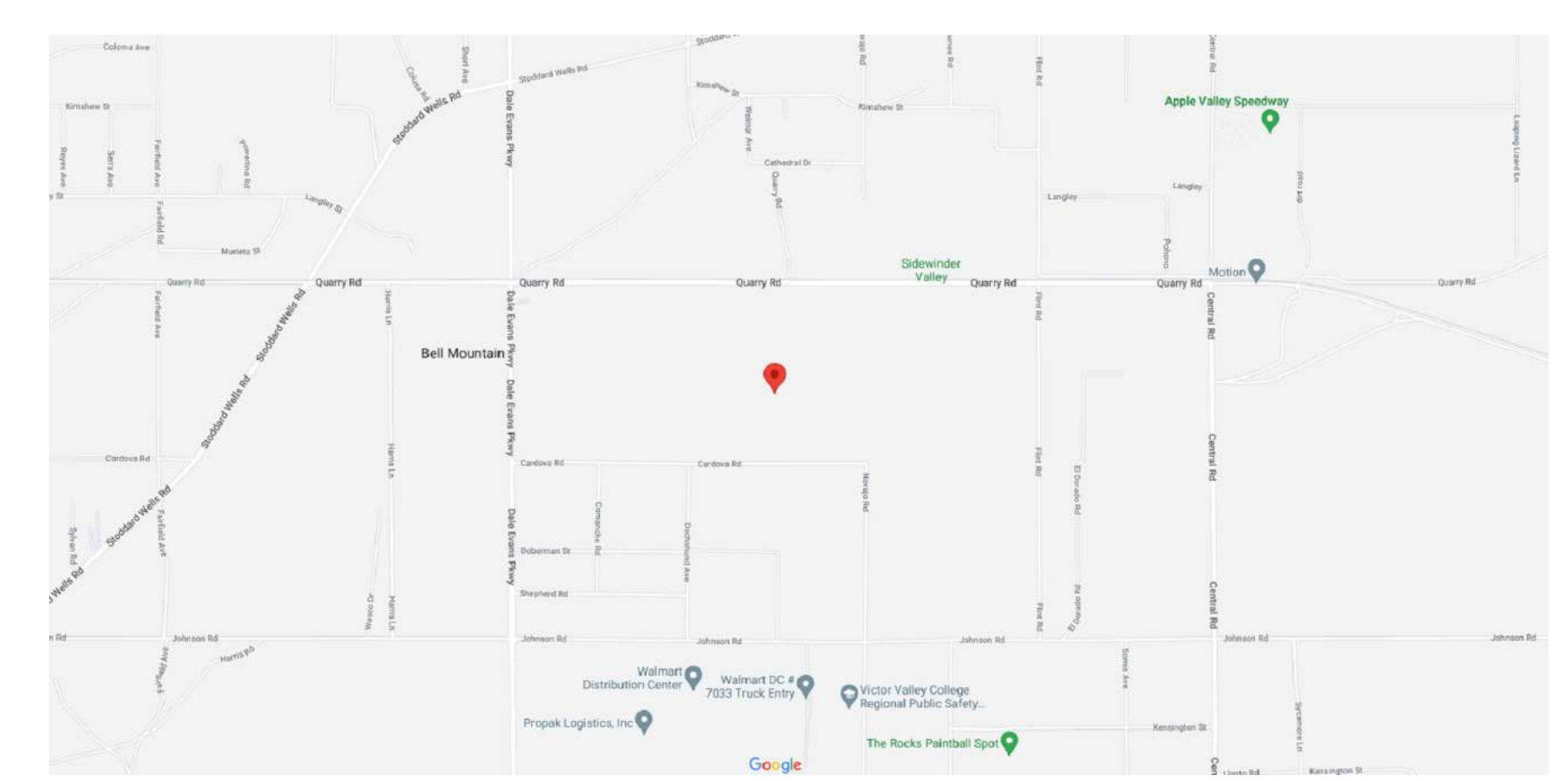


INDUSTRIAL BLDG
 FOOTPRINT: 1,346,270 SF
 MEZZ: 26,880 SF
 GFA: 1,373,150 SF
 CLR. HGT: 40'

DEVELOPMENT STANDARDS		
Zoning		
Jurisdiction	Apple Valley, CA	
Zoning Designation	I-SP (Specific Plan)	
Max Coverage	45% ⁴	
Max Height	100 FT	
Building Setbacks	1	
Landscape Setbacks	4	
Parking Standards		
	Min Stall Size	9X19
	Drive Aisle	24 FT
Required Parking		
	Office	1/250 SF ⁵
	Manufacturing	1/500 SF ³
	Non-Specified	3/1000 SF ⁶
	Warehouse	1/500 SF ²
	<10000 SF	1/500 SF
	10000≥# SF	1/1000 SF

PROJECT DATA		
Site Summary		
Gross Site Area	3,927,102 SF	90.15 AC
Total Building Area(s)	Gross Floor Area	1,373,150 SF
	Footprint	1,346,270 SF
	Coverage	Gross 34%
	FAR	Gross 0.35
Building 1		
Building Area(s)	Footprint	1,346,270 SF
	Mezzanine	26,880 SF
	Gross Floor Area	1,373,150 SF
	@4% Office	1,383 Stalls
Cars Required	@1.05/1,000 SF	1,440 Stalls
Cars Provided	Req. Accessible	29 Stalls
Drive-in Doors		4
Docks	@1.63/10,000 SF	224
Trailers		619 Stalls

- Notes**
- From major or secondary streets, the setback is 50 FT. From local industrial streets, the setback is 25 FT.
 - 1 space per 500 SF of GFA for the first 10,000 SF and beyond that, 1 space per 1,000 SF of GFA; for office area that exceeds 25%, provide 1 space per 200 SF in excess of 25%
 - 1 space per 500 SF of GFA or 1 space per 2 employees on the largest shift, whichever is greater; for office area that exceeds 25%, provide 1 space per 200 SF in excess of 25%
 - To be determined by city.
 - Minimum required number of parking spaces: 4
 - 3 spaces per 1000 SF of GFA or 1 space per 2 employees on the largest shift, whichever is greater; for office area that exceeds 25%, provide 1 space per 200 SF in excess of 25%



This conceptual design is based upon a preliminary review of entitlement requirements and on unverified and possibly incomplete site and/or building information, and is intended merely to assist in exploring how the project might be developed.

Boundary Source: PDF ALTA SURVEY

Stormwater Management Design:

SCHEME: 03

Conceptual Site Plan
 Redwood West Cordova Apple Valley
 Cardova Rd, Apple Valley, CA 92307

WARE MALCOMB

IRV23-0142-00
 2023.08.02
 PAGE 01

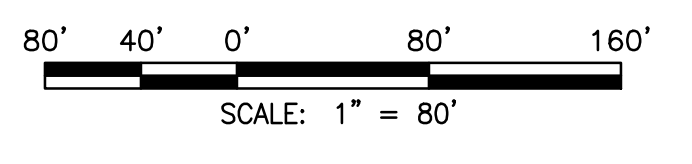
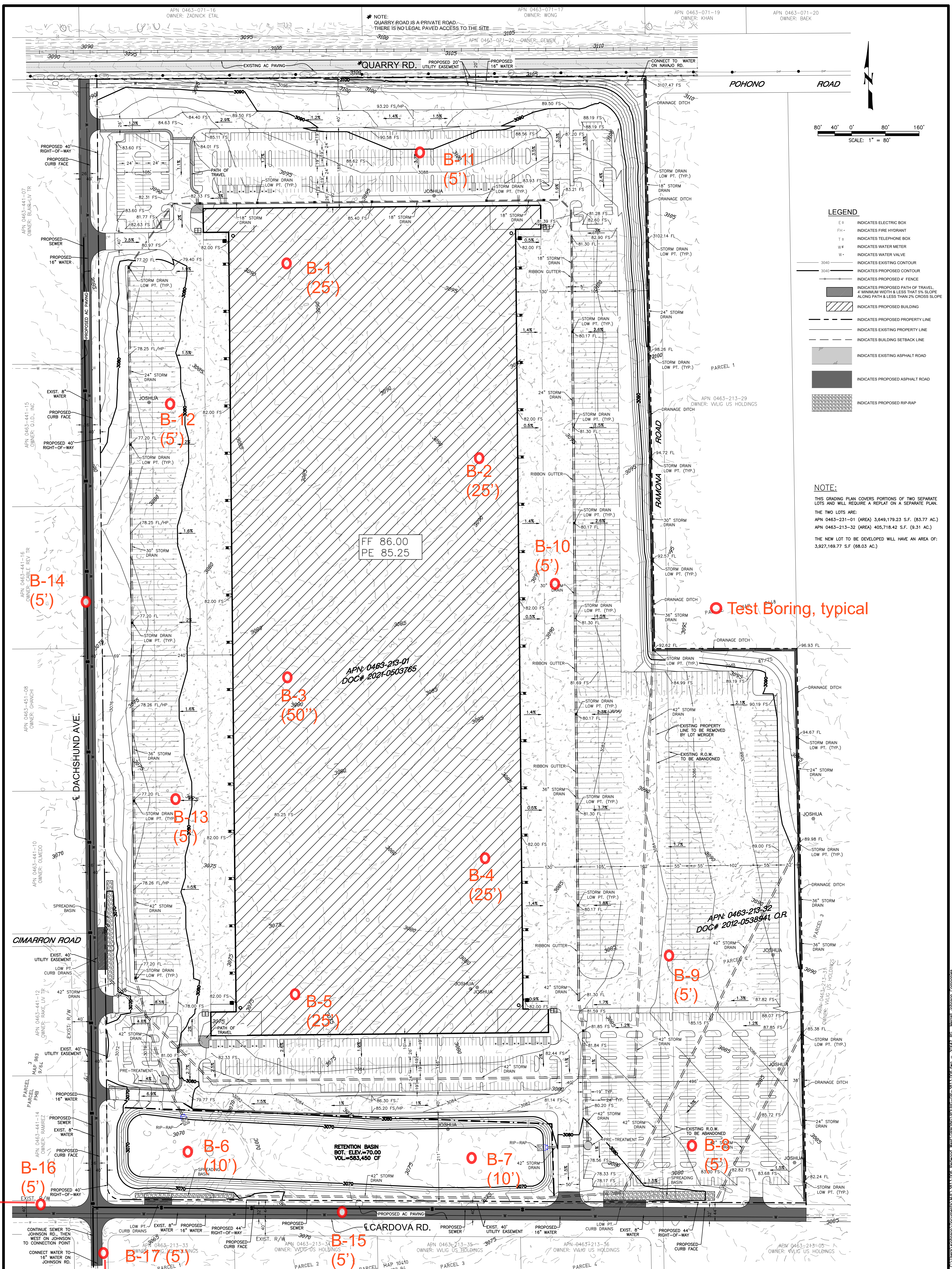
APN 0463-071-16
OWNER: ZADNICK ETAL

APN 0463-071-17
OWNER: WONG

APN 0463-071-19
OWNER: KHAN

APN 0463-071-20
OWNER: BAEK

* NOTE:
QUARRY ROAD IS A PRIVATE ROAD
THERE IS NO LEGAL PAVED ACCESS TO THE SITE



LEGEND

- E-B INDICATES ELECTRIC BOX
- FH+ INDICATES FIRE HYDRANT
- T-B INDICATES TELEPHONE BOX
- WM INDICATES WATER METER
- W+ INDICATES WATER VALVE
- 3040 INDICATES EXISTING CONTOUR
- 3040 INDICATES PROPOSED CONTOUR
- INDICATES PROPOSED 4' FENCE
- INDICATES PROPOSED PATH OF TRAVEL, 4' MINIMUM WIDTH & LESS THAN 2% SLOPE ALONG PATH & LESS THAN 2% CROSS SLOPE
- INDICATES PROPOSED BUILDING
- INDICATES PROPOSED PROPERTY LINE
- INDICATES EXISTING PROPERTY LINE
- INDICATES BUILDING SETBACK LINE
- INDICATES EXISTING ASPHALT ROAD
- INDICATES PROPOSED ASPHALT ROAD
- INDICATES PROPOSED RIP-RAP

NOTE:

THIS GRADING PLAN COVERS PORTIONS OF TWO SEPARATE LOTS AND WILL REQUIRE A REPEAT ON A SEPARATE PLAN. THE TWO LOTS ARE:
 APN 0463-231-01 (AREA) 3,649,179.23 S.F. (83.77 AC.)
 APN 0463-231-32 (AREA) 405,718.42 S.F. (9.31 AC.)
 THE NEW LOT TO BE DEVELOPED WILL HAVE AN AREA OF: 3,927,169.77 S.F. (90.08 AC.)

Test Boring, typical

NOT FOR CONSTRUCTION

OWNER:
 BUTTERFLY EQUITY PARTNERS AND/OR
 RW APPLEVALLEY II AND/OR LINKUP, LLC
 220 NEWPORT CENTER DRIVE, STE 11-557
 NEWPORT BEACH, CA 92660

DATE	DELTA	REVISION DESCRIPTION	APPROVAL DATE	BY

MERRELL JOHNSON

MERRELL JOHNSON ENGINEERING, INC.
 22221 U.S. HIGHWAY 8, APPLE VALLEY, CA 92307
 760 240 8000 | MERRELLJOHNSON.COM

**CONCEPTUAL GRADING PLAN
 WAREHOUSE
 CARDOVA RD & DACHSHUND AVE.
 68.2 ACRES**

FOR
REDWOOD WEST

DRAWN BY: EKI
 DATE: 9/18/23
 JOB NO. 3813.006
 SHEET 1 OF 1

APPENDIX B

Exploratory Logs

Soil Classification Key

Unified Soil Classification System (USCS) and Particle Size Limits

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: B
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Unified Soil Classification System (USCS)

Soil Type	Description	Symbol	Soil Description	
			Symbol	Description
Coarse Grained Soils More Than 50% Is Larger Than No. 200 Sieve	Gravel and Gravelly Soils More Than 50% Retained on No. 4 Sieve	Clean Gravels Little Or No Fines	GW	Well-graded gravels, gravel-sand mixtures, little or no fines
			GP	Poorly-graded gravels, gravel-sand mixtures, little or no fines
		Gravels w/ Fines Appreciable Amount	GM	Silty gravels, gravel-sand-silt mixtures
			GC	Clayey gravels, gravel-sand-clay mixtures
	Sand and Sandy Soils More Than 50% Passing No. 4 Sieve	Clean Sand Little Or No Fines	SW	Well-graded sands, gravelly sands, little or no fines
			SP	Poorly-graded sands, gravelly sands, little or no fines
		Sands w/ Fines Appreciable Amount	SM	Silty-sands, sand-silt mixtures
			SC	Clayey sands, sand-clay mixtures
Fine Grained Soils More Than 50% Is Smaller Than No. 200 Sieve	Silts and Clays Liquid Limit Less Than 50	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity	
		CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays	
		OL	Organic silts and organic silty clays of low plasticity	
	Silts and Clays Liquid Limit Greater Than 50	MH	Inorganic silts, micaceous or diatomaceous fine sand or silty soils	
		CH	Inorganic clays of high plasticity, fat clays	
		OH	Organic clays of medium to high plasticity, organic silts	
	Highly Organic Soils	PT	Peat, humus, swamp soils with high organic contents	

Particle Size Limits

Division	Silt or Clay	Sand			Gravel		Cobbles	Boulders
		Fine	Medium	Coarse	Fine	Coarse		
U.S. Sieve	No. 200	No. 40	No. 10	No. 4	3/4"	3"	12"	
Grain (mm)	0.075	0.420	2.00	4.76	19.1	76.2	305	

Soils possessing characteristics of two classifications are designated by group symbol combination. Soils may be classified initially using the visual manual procedure prior to laboratory test.



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Exploratory Log


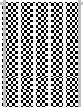

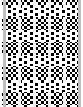
ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
 Sheet: 1 of 2
 Appendix: B
 Permit No:
 Client Project No:
 USA Ticket No:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Location No: B1 Start Date/Time: 10/25/23 0730 End Date/Time: 10/25/23 0825

Conducted By: J. Alborno
 Operator: A. Lara
 Equipment Type: CME-75-HSA
 Drive Weight (lb): 140
 Drive Drop (in): 30
 Excavation Type: Auger Hole
 Dimensions: 8" x 50'
 Advance Assist: None
 Field Tests: D3550
 Shoring Type: None
 Elevation: 2868
 Groundwater: Not Encountered
 Recent Weather: Clear
 Sampler Insertion: Driven
 Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
0	20, 21, 26		4.1	95.4	SM		Light Brown, Dry, Dense, Silty Sand Bulk Sample at 0' to 5' - JDA10252301	SA, MD, DS
	30, 32, 50		4.6	96.6			Tube Sample at 1' - JDA10252302	TD
	32, 50 (3")		3.2	99.3			Tube Sample at 3' - JDA10252303	TD
5	50 (3")				SPSM		Tube Sample at 5' - JDA10252304 Light Brown, Dry, Very Dense, Poorly Graded Sand with Silt	TD
10	50 (4")		2.8	104.8			Tube Sample at 10' - JDA10252305 (One Tube Recovered)	TD
15	50 (5")					SPT at 15'		
20	50 (4")					SPT at 20', No Recovery		
25	50 (3")					SPT at 25', No recovery Difficulty Drilling		

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

(1)  =Bulk (2) **DS** =Direct Shear **SA** =Sieve Analysis **MD** =Max Density **AL** =Atterberg Limits **CN** =Consolidation
 =Driven **EI** =Expansion Index **CR** =Corrosion **RV** =R-Value **SE** =Sand Equivalent **TD** =Tube Density



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Exploratory Log

ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
Sheet: 2 of 2
Appendix: B
Permit No:
Client Project No:
USA Ticket No:
DSA File No:
DSA Application No:
DSA LEA No:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Location No: B1 (Cont.) **Start Date/Time:** 10/25/23 0730 **End Date/Time:** 10/25/23 0825

Conducted By: J. Albormoz	Excavation Type: Auger Hole	Elevation: 2868
Operator: A. Lara	Dimensions: 8" x 50'	Groundwater: Not Encountered
Equipment Type: CME-75 HSA	Advance Assist: None	Recent Weather: Clear
Drive Weight (lb): 140	Field Tests: D3550	Sampler Insertion: Driven
Drive Drop (in): 30	Shoring Type: None	Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
30	50 (2")				SPSM		SPT at 30' Boulder/Rock Encountered	
35								
40	50 (3")						SPT at 40'	
45							Very Difficult Drilling	
50	50 (2")						SPT at 50' *Drilling Terminated at Approximately 50*	
55								

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

(1) =Bulk	(2) DS =Direct Shear	SA =Sieve Analysis	MD =Max Density	AL =Atterberg Limits	CN =Consolidation
 =Driven	EI =Expansion Index	CR =Corrosion	RV =R-Value	SE =Sand Equivalent	TD =Tube Density



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Exploratory Log

ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: B
 Permit No:
 Client Project No:
 USA Ticket No:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Location No: B2 Start Date/Time: 10/25/23 0920 End Date/Time: 10/25/2023 0950

Conducted By: J. Alborno
 Operator: A. Lara
 Equipment Type: CME-75 HSA
 Drive Weight (lb): 140
 Drive Drop (in): 30
 Excavation Type: Auger Hole
 Dimensions: 8" x 25'
 Advance Assist: None
 Field Tests: D3550
 Shoring Type: None
 Elevation: 2869
 Groundwater: Not Encountered
 Recent Weather: Clear
 Sampler Insertion: Driven
 Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
0	10, 10		3.4	94.2	SM		Light Brown, Dry, Medium Dense, Silty Sand Bulk Sample at 0' to 5' - JDA10252306	SA
	11						Tube Sample at 1' - JDA10252307	TD
	10, 12		2.5	107.2			Tube Sample at 3' - JDA10252308	TD
	18							
5	27,		3.9	96.7			Tube Sample at 5' - JDA10252309	TD
	50 (3")						Very Dense Bulk Sample at 5' to 10' - JDA10252310	SA, CN, CR
					SPSM		Light Brown, Dry, Very Dense, Poorly Graded Sand with Silt	
10	50 (3")						Tube Sample at 10' - No Recovery	
15	50 (3")						SPT at 15' Bedrock Encountered	
20	50 (3")						SPT at 20'	
25	50 (4")						SPT at 25' *Drilling Terminated at Approximately 25*	

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

(1) =Bulk (2) **DS** =Direct Shear **SA** =Sieve Analysis **MD** =Max Density **AL** =Atterberg Limits **CN** =Consolidation
 =Driven **EI** =Expansion Index **CR** =Corrosion **RV** =R-Value **SE** =Sand Equivalent **TD** =Tube Density



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Exploratory Log

ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: B
 Permit No:
 Client Project No:
 USA Ticket No:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Location No: B3 Start Date/Time: 10/25/23 1010 End Date/Time: 10/25/23 1040

Conducted By: J. Alborno
 Operator: A. Lara
 Equipment Type: CME-75 HSA
 Drive Weight (lb): 140
 Drive Drop (in): 30
 Excavation Type: Auger Hole
 Dimensions: 8" x 25'
 Advance Assist: None
 Field Tests: D3550
 Shoring Type: None
 Elevation: 2863
 Groundwater: Not Encountered
 Recent Weather: Clear
 Sampler Insertion: Driven
 Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
0	35, 50 (5")		4.3	98.0			Light Brown, Dry, Very Dense, Silty Sand Bulk Sample at 0' to 5' - JDA10252311	MD, CR, EI
	33, 30, 34		7.7	84.3			Tube Sample at 1' - JDA10252312	TD
	19, 26, 39		6.5	85.2			Whitish brown Tube Sample at 3' - JDA10252313	TD
5							Tube Sample at 5' - JDA10252314	TD
10	33, 50 (4")		2.9	97.5			Tube Sample at 10' - JDA10252315	TD
							Light Brown, Dry, Very Dense, Poorly Graded Sand with Silt	
15	50 (4")						SPT at 15'	
20	50 (4")						SPT at 20'	
25	50 (3")						SPT at 25' - No Recovery *Drilling Terminated at Approximately 25*	

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

(1) =Bulk (2) **DS** =Direct Shear **SA** =Sieve Analysis **MD** =Max Density **AL** =Atterberg Limits **CN** =Consolidation
 =Driven **EI** =Expansion Index **CR** =Corrosion **RV** =R-Value **SE** =Sand Equivalent **TD** =Tube Density



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Exploratory Log
















ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: B
 Permit No:
 Client Project No:
 USA Ticket No:
 DSA File No:
 DSA Application No:
 DSA LEA No:


Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Location No: B4 Start Date/Time: 10/25/23 1100 End Date/Time: 10/25/2023 1130

Conducted By: J. Albormoz Excavation Type: Auger Hole Elevation: 2861
 Operator: A. Lara Dimensions: 8" x 25' Groundwater: Not Encountered
 Equipment Type: CME-75 HSA Advance Assist: None Recent Weather: Clear
 Drive Weight (lb): 140 Field Tests: D3550 Sampler Insertion: Driven
 Drive Drop (in): 30 Shoring Type: None Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
0	50 (6")		2.6	102.0			Light Brown, Dry, Poorly Graded Sand with Silt Bulk Sample at 0' to 5' - JDA10252315	MD, DS TD
	50 (4")						Tube Sample at 1' - JDA10252316 (One Tube Recovered)	
	50 (2")						Tube Sample at 3' - No Recovery	
5	50 (2")						Tube Sample at 5' - No Recovery	
							Grayish Brown, Poorly Graded Sand	
							Bedrock Encountered	
10	50 (3")						Tube Sample at 10' - No Recovery	
15	50 (4")						SPT at 15'	
20	50 (3")						SPT at 20'	
25	50 (4")						SPT at 25' - No Recovery	
							*Drilling Terminated at Approximately 25'	

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

⁽¹⁾  =Bulk  =Driven ⁽²⁾ DS =Direct Shear EI =Expansion Index SA =Sieve Analysis CR =Corrosion MD =Max Density RV =R-Value AL =Atterberg Limits SE =Sand Equivalent CN =Consolidation TD =Tube Density



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Exploratory Log

ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
Sheet: 1 of 1
Appendix: B
Permit No:
Client Project No:
USA Ticket No:
DSA File No:
DSA Application No:
DSA LEA No:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Location No: B5 **Start Date/Time:** 10/25/23 1155 **End Date/Time:** 10/25/23 1220

Conducted By: J. Alborno	Excavation Type: Auger Hole	Elevation: 2863
Operator: A. Lara	Dimensions: 8" x 25'	Groundwater: Not Encountered
Equipment Type: CME-75 HSA	Advance Assist: None	Recent Weather: Clear
Drive Weight (lb): 140	Field Tests: D3550	Sampler Insertion: Driven
Drive Drop (in): 30	Shoring Type: None	Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
0	50 (6")		3.7	104.6			Light Brown, Dry, Silty Sand Bulk Sample at 0' to 5' - JDA10252317 Tube Sample at 1' - JDA10252318	SA TD
5	50 (4")		2.8	101.6			Tube Sample at 5' - JDA102523219 (One Tube Recovered) Light Brown, Dry, Poorly Graded Sand with Silt Bedrock Encountered	TD
10	50 (6")		4.2	96.8			Tube Sample at 10' - JDA10252320 (One Tube Recovered)	TD
15	50 (3")						SPT at 15' Difficulty Drilling	
20	50 (3")						SPT at 20' - No Recovery	
25	50 (3")						SPT at 25' *Drilling Terminated at Approximately 25*	

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

(1) =Bulk	(2) DS =Direct Shear	SA =Sieve Analysis	MD =Max Density	AL =Atterberg Limits	CN =Consolidation
 =Driven	EI =Expansion Index	CR =Corrosion	RV =R-Value	SE =Sand Equivalent	TD =Tube Density



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Exploratory Log

ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
Sheet: 1 of 1
Appendix: B
Permit No:
Client Project No:
USA Ticket No:
DSA File No:
DSA Application No:
DSA LEA No:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Location No: B6 (Perc.) **Start Date/Time:** 10/25/23 1240 **End Date/Time:** 10/25/23 1300

Conducted By: J. Albormoz	Excavation Type: Auger Hole	Elevation: 2862
Operator: A. Lara	Dimensions: 8" x 10'	Groundwater: Not Encountered
Equipment Type: CME-75 HSA	Advance Assist: None	Recent Weather: Clear
Drive Weight (lb): 140	Field Tests: D3550	Sampler Insertion: Driven
Drive Drop (in): 30	Shoring Type: None	Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
0							Light Brown, Dry, Poorly Graded Sand with Silt	
5							Bulk Sample at 5' to 10' - JDA10252321	SA
10							*Drilling Terminated at Approximately 10'	
15								
20								
25								

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

(1) =Bulk	(2) DS =Direct Shear	SA =Sieve Analysis	MD =Max Density	AL =Atterberg Limits	CN =Consolidation
 =Driven	EI =Expansion Index	CR =Corrosion	RV =R-Value	SE =Sand Equivalent	TD =Tube Density



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Exploratory Log

ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
Sheet: 1 of 1
Appendix: B
Permit No:
Client Project No:
USA Ticket No:
DSA File No:
DSA Application No:
DSA LEA No:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Location No: B7 (Perc.) **Start Date/Time:** 10/25/23 1315 **End Date/Time:** 10/25/23 1330

Conducted By: J. Albormoz	Excavation Type: Auger Hole	Elevation: 2863
Operator: A. Lara	Dimensions: 8" x 10'	Groundwater: Not Encountered
Equipment Type: CME-75 HSA	Advance Assist: None	Recent Weather: Clear
Drive Weight (lb): 140	Field Tests: D3550	Sampler Insertion: Driven
Drive Drop (in): 30	Shoring Type: None	Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
0							Light Brown, Dry, Poorly Graded Sand with Silt Rock Encountered	
5							Bulk Sample 5' to 10' - JDA10252322	SA
10							*Drilling Terminated at Approximately 10**	
15								
20								
25								

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

(1) =Bulk	(2) DS =Direct Shear	SA =Sieve Analysis	MD =Max Density	AL =Atterberg Limits	CN =Consolidation
 =Driven	EI =Expansion Index	CR =Corrosion	RV =R-Value	SE =Sand Equivalent	TD =Tube Density



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Exploratory Log

ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
Sheet: 1 of 1
Appendix: B
Permit No:
Client Project No:
USA Ticket No:
DSA File No:
DSA Application No:
DSA LEA No:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Location No: B8 **Start Date/Time:** 10/25/23 1335 **End Date/Time:** 10/25/23 1345

Conducted By: J. Albormoz	Excavation Type: Auger Hole	Elevation: 2862
Operator: A. Lara	Dimensions: 8" x 5'	Groundwater: Not Encountered
Equipment Type: CME-75 HSA	Advance Assist: None	Recent Weather: Clear
Drive Weight (lb): 140	Field Tests: D3550	Sampler Insertion: Driven
Drive Drop (in): 30	Shoring Type: None	Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
0							Light Brown, Dry, Poorly Graded Sand with Silt Bulk Sample at 0' to 5' - JDA10252323	SA, SE
5							*Drilling Terminated at Approximately 5*	
10								
15								
20								
25								

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

(1) =Bulk	(2) DS =Direct Shear	SA =Sieve Analysis	MD =Max Density	AL =Atterberg Limits	CN =Consolidation
 =Driven	EI =Expansion Index	CR =Corrosion	RV =R-Value	SE =Sand Equivalent	TD =Tube Density



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Exploratory Log

ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
Sheet: 1 of 1
Appendix: B
Permit No:
Client Project No:
USA Ticket No:
DSA File No:
DSA Application No:
DSA LEA No:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Location No: B9 **Start Date/Time:** 10/25/23 1350 **End Date/Time:** 10/25/23 1355

Conducted By: J. Albormoz	Excavation Type: Auger Hole	Elevation: 2865
Operator: A. Lara	Dimensions: 8" x 5'	Groundwater: Not Encountered
Equipment Type: CME-75 HSA	Advance Assist: None	Recent Weather: Clear
Drive Weight (lb): 140	Field Tests: D3550	Sampler Insertion: Driven
Drive Drop (in): 30	Shoring Type: None	Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
0							Light Brown, Dry, Silty Sand Bulk Sample at 0' to 5' - JDA10252324	SA, SE
5							*Drilling Terminated at Approximately 5'	
10								
15								
20								
25								

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

(1) =Bulk	(2) DS =Direct Shear	SA =Sieve Analysis	MD =Max Density	AL =Atterberg Limits	CN =Consolidation
 =Driven	EI =Expansion Index	CR =Corrosion	RV =R-Value	SE =Sand Equivalent	TD =Tube Density



engineering | surveying | testing | inspection

Exploratory Log

ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: B
 Permit No:
 Client Project No:
 USA Ticket No:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Location No: B10 Start Date/Time: 10/25/23 1400 End Date/Time: 10/25/23 1405

Conducted By: J. Albormoz Excavation Type: Auger Hole Elevation: 2864
 Operator: A. Lara Dimensions: 8" x 5' Groundwater: Not Encountered
 Equipment Type: CME-75 HSA Advance Assist: None Recent Weather: Clear
 Drive Weight (lb): 140 Field Tests: D3550 Sampler Insertion: Driven
 Drive Drop (in): 30 Shoring Type: None Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
0							Whitish Brown, Dry, Silty Sand with Gravel Bulk Sample at 0' to 5' - JDA10252325	SA, SE, RV
5							*Drilling Terminated at Approximately 5*	
10								
15								
20								
25								

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

(1) =Bulk =Driven
 (2) **DS** =Direct Shear **EI** =Expansion Index **SA** =Sieve Analysis **CR** =Corrosion **MD** =Max Density **RV** =R-Value **AL** =Atterberg Limits **SE** =Sand Equivalent **CN** =Consolidation **TD** =Tube Density



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Exploratory Log

ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
Sheet: 1 of 1
Appendix: B
Permit No:
Client Project No:
USA Ticket No:
DSA File No:
DSA Application No:
DSA LEA No:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Location No: B11 **Start Date/Time:** 10/25/23 1407 **End Date/Time:** 10/25/23 1410

Conducted By: J. Albormoz	Excavation Type: Auger Hole	Elevation: 2867
Operator: A. Lara	Dimensions: 8" x 5'	Groundwater: Not Encountered
Equipment Type: CME-75 HSA	Advance Assist: None	Recent Weather: Clear
Drive Weight (lb): 140	Field Tests: D3550	Sampler Insertion: Driven
Drive Drop (in): 30	Shoring Type: None	Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
0							Whitish Brown, Dry, Silty Sand with Gravel Bulk Sample 0' to 5' - JDA10252326	SA, SE
5							*Drilling Terminated at Approximately 5*	
10								
15								
20								
25								

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

(1) =Bulk	(2) DS =Direct Shear	SA =Sieve Analysis	MD =Max Density	AL =Atterberg Limits	CN =Consolidation
 =Driven	EI =Expansion Index	CR =Corrosion	RV =R-Value	SE =Sand Equivalent	TD =Tube Density



engineering | surveying | testing | inspection

Exploratory Log

ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
Sheet: 1 of 1
Appendix: B
Permit No:
Client Project No:
USA Ticket No:
DSA File No:
DSA Application No:
DSA LEA No:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Location No: B12 **Start Date/Time:** 10/25/23 1416 **End Date/Time:** 10/25/23 1420

Conducted By: J. Albormoz	Excavation Type: Auger Hole	Elevation: 2864
Operator: A. Lara	Dimensions: 8" x 5'	Groundwater: Not Encountered
Equipment Type: CME-75 HSA	Advance Assist: None	Recent Weather: Clear
Drive Weight (lb): 140	Field Tests: D3550	Sampler Insertion: Driven
Drive Drop (in): 30	Shoring Type: None	Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
0							Light Brown, Dry, Silty Sand Bulk Sample at 0' to 5' - JDA10252327	SA, SE
5							*Drilling Terminated at Approximately 5*	
10								
15								
20								
25								

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

(1) =Bulk	(2) DS =Direct Shear	SA =Sieve Analysis	MD =Max Density	AL =Atterberg Limits	CN =Consolidation
 =Driven	EI =Expansion Index	CR =Corrosion	RV =R-Value	SE =Sand Equivalent	TD =Tube Density



engineering | surveying | testing | inspection

Exploratory Log

ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: B
 Permit No:
 Client Project No:
 USA Ticket No:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Location No: B13 Start Date/Time: 10/25/23 1425 End Date/Time: 10/25/23 1430

Conducted By: J. Alborno
 Operator: A. Lara
 Equipment Type: CME-75
 Drive Weight (lb): 140
 Drive Drop (in): 30
 Excavation Type: Auger Hole
 Dimensions: 8" x 5'
 Advance Assist: None
 Field Tests: D3550
 Shoring Type: None
 Elevation: 2862
 Groundwater: Not Encountered
 Recent Weather: Clear
 Sampler Insertion: Driven
 Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
0							Light Brown, Dry, Silty Sand Bulk Sample at 0' to 5' - JDA10252328	SA, SE
5							*Drilling Terminated at Approximately 5*	
10								
15								
20								
25								

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

(1) =Bulk =Driven
 (2) **DS** =Direct Shear **EI** =Expansion Index **SA** =Sieve Analysis **CR** =Corrosion **MD** =Max Density **RV** =R-Value **AL** =Atterberg Limits **SE** =Sand Equivalent **CN** =Consolidation **TD** =Tube Density



engineering | surveying | testing | inspection

Exploratory Log

ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
Sheet: 1 of 1
Appendix: B
Permit No:
Client Project No:
USA Ticket No:
DSA File No:
DSA Application No:
DSA LEA No:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Location No: B14 Dachshund Avenue **Start Date/Time:** 10/25/23 1425 **End Date/Time:** 10/25/23 1430

Conducted By: J. Alborno	Excavation Type: Auger Hole	Elevation: 2862
Operator: A. Lara	Dimensions: 8" x 5'	Groundwater: Not Encountered
Equipment Type: CME-75	Advance Assist: None	Recent Weather: Clear
Drive Weight (lb): 140	Field Tests: D3550	Sampler Insertion: Driven
Drive Drop (in): 30	Shoring Type: None	Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
0							Light Brown, Dry, Silty Sand Bulk Sample at 0' to 5' - JDA10252328	SA, SE
5							*Drilling Terminated at Approximately 5*	
10								
15								
20								
25								

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

(1) =Bulk	(2) DS =Direct Shear	SA =Sieve Analysis	MD =Max Density	AL =Atterberg Limits	CN =Consolidation
 =Driven	EI =Expansion Index	CR =Corrosion	RV =R-Value	SE =Sand Equivalent	TD =Tube Density



engineering | surveying | testing | inspection

Exploratory Log

ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
Sheet: 1 of 1
Appendix: B
Permit No:
Client Project No:
USA Ticket No:
DSA File No:
DSA Application No:
DSA LEA No:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Location No: B15, Cardova Road **Start Date/Time:** 10/25/23 1425 **End Date/Time:** 10/25/23 1430

Conducted By: J. Alborno	Excavation Type: Auger Hole	Elevation: 2862
Operator: A. Lara	Dimensions: 8" x 5'	Groundwater: Not Encountered
Equipment Type: CME-75	Advance Assist: None	Recent Weather: Clear
Drive Weight (lb): 140	Field Tests: D3550	Sampler Insertion: Driven
Drive Drop (in): 30	Shoring Type: None	Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
0							Light Brown, Dry, Silty Sand Bulk Sample at 0' to 5' - JDA10252328	SA, SE
5							*Drilling Terminated at Approximately 5*	
10								
15								
20								
25								

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

(1) =Bulk	(2) DS =Direct Shear	SA =Sieve Analysis	MD =Max Density	AL =Atterberg Limits	CN =Consolidation
 =Driven	EI =Expansion Index	CR =Corrosion	RV =R-Value	SE =Sand Equivalent	TD =Tube Density



engineering | surveying | testing | inspection

Exploratory Log

ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: B
 Permit No:
 Client Project No:
 USA Ticket No:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Location No: B16, Cardova Road Start Date/Time: 10/25/23 1425 End Date/Time: 10/25/23 1430

Conducted By: J. Alborno Excavation Type: Auger Hole Elevation: 2862
 Operator: A. Lara Dimensions: 8" x 5' Groundwater: Not Encountered
 Equipment Type: CME-75 Advance Assist: None Recent Weather: Clear
 Drive Weight (lb): 140 Field Tests: D3550 Sampler Insertion: Driven
 Drive Drop (in): 30 Shoring Type: None Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
0							Light Brown, Dry, Silty Sand Bulk Sample at 0' to 5' - JDA10252328	SA, SE
5							*Drilling Terminated at Approximately 5*	
10								
15								
20								
25								

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

(1) =Bulk (2) **DS** =Direct Shear **SA** =Sieve Analysis **MD** =Max Density **AL** =Atterberg Limits **CN** =Consolidation
 =Driven **EI** =Expansion Index **CR** =Corrosion **RV** =R-Value **SE** =Sand Equivalent **TD** =Tube Density



engineering | surveying | testing | inspection

Exploratory Log

ASTM D5434, D1452, D1586, D1587, D2488 (USCS), D3550

Report Date: 12/01/23
Sheet: 1 of 1
Appendix: B
Permit No:
Client Project No:
USA Ticket No:
DSA File No:
DSA Application No:
DSA LEA No:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Location No: B17, Dachshund Avenue **Start Date/Time:** 10/25/23 1425 **End Date/Time:** 10/25/23 1430

Conducted By: J. Alborno	Excavation Type: Auger Hole	Elevation: 2862
Operator: A. Lara	Dimensions: 8" x 5'	Groundwater: Not Encountered
Equipment Type: CME-75	Advance Assist: None	Recent Weather: Clear
Drive Weight (lb): 140	Field Tests: D3550	Sampler Insertion: Driven
Drive Drop (in): 30	Shoring Type: None	Preservation: D4220

Depth (ft)	'N' Value	Sample ⁽¹⁾	Moisture (%)	Density (pcf)	Class (USCS)	Graphic	Description / Comments	Lab Tests ⁽²⁾
0							Light Brown, Dry, Silty Sand Bulk Sample at 0' to 5' - JDA10252328	SA, SE
5							*Drilling Terminated at Approximately 5*	
10								
15								
20								
25								

Comments: "N" Value Based on 2.5" diameter modified California Tube Sampler (ASTM D3550) or SPT (ASTM D1586) as noted on log. Some boulder/rock encountered during drilling operations. Partial caving of hole observed.

(1) =Bulk	(2) DS =Direct Shear	SA =Sieve Analysis	MD =Max Density	AL =Atterberg Limits	CN =Consolidation
 =Driven	EI =Expansion Index	CR =Corrosion	RV =R-Value	SE =Sand Equivalent	TD =Tube Density



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APPENDIX C

Laboratory Testing

Particle-Size Analysis of Soil

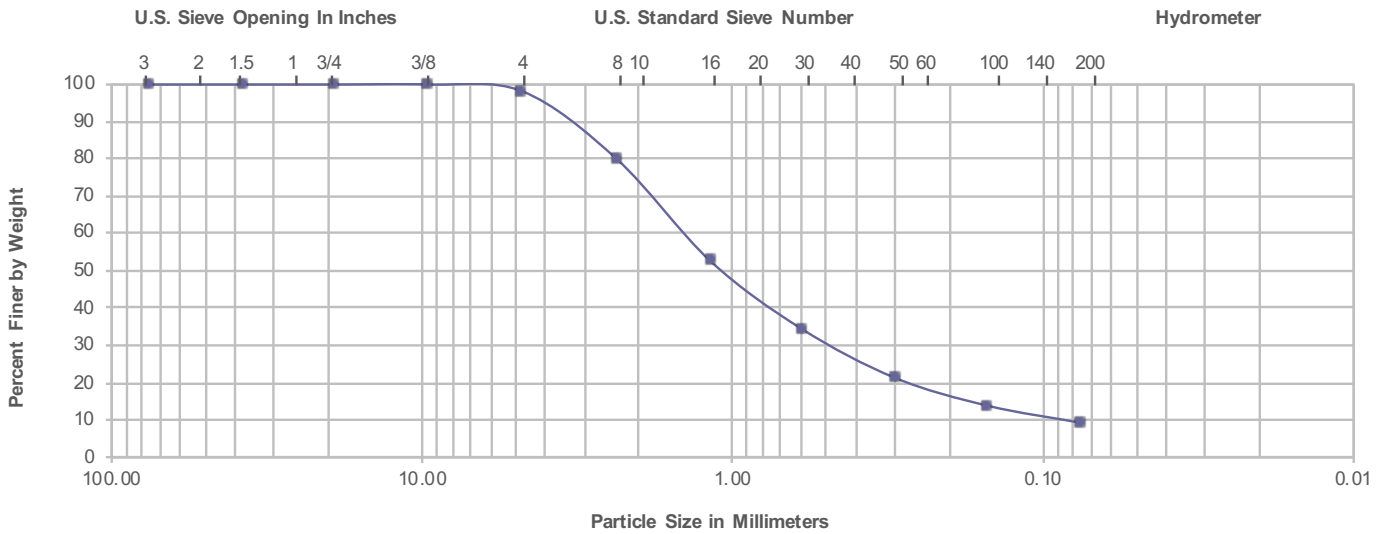
D422, D1140, D2487

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252310 Gravel (%): 1.8% Sand (%): 89.0% Fines (%): 9.2%

Classification, ASTM D2487: (SPSM) Poorly graded sand with silt
 Sample Origin: Boring Two at 5' to 10'
 Laboratory Remarks:



C _u	C _c	Moisture	D ₁₀₀	D ₆₀	D ₃₀	D ₁₀	LL	PL	PI	SG	FM	SE
17	2	3.1%	5.231	1.498	0.499	0.089	ND	ND	ND	ND	ND	ND

Method / Procedure Used: D422, D1140
 Size of Initial Dry Mass (g): 509.0
 Determination of Dry Mass: D2216
 Particles; Shape, Hardness: ND
 Dispersion Device/Period: Manual/2 hr
 Type & Amount of Agent: Defloc. & 1.0
 Laboratory Comments:

The Material Was Met Was Not Met Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District



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Particle-Size Analysis of Soil

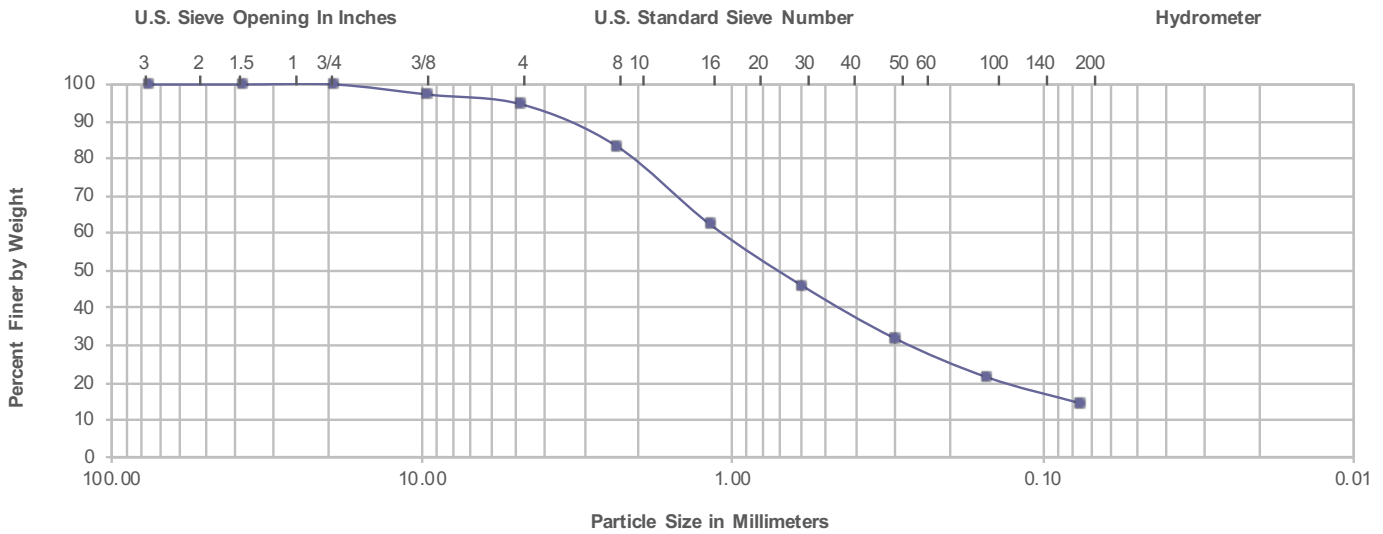
D422, D1140, D2487

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252317 Gravel (%): 5.4% Sand (%): 80.3% Fines (%): 14.3%

Classification, ASTM D2487: (SM) Silty sand
 Sample Origin: Boring Five at 0' to 5'
 Laboratory Remarks:



C _u	C _c	Moisture	D ₁₀₀	D ₆₀	D ₃₀	D ₁₀	LL	PL	PI	SG	FM	SE
NA	NA	4.0%	15.519	1.095	0.274	0.000	ND	ND	ND	ND	ND	ND

Method / Procedure Used: D422, D1140
 Size of Initial Dry Mass (g): 483.6
 Determination of Dry Mass: D2216
 Particles; Shape, Hardness: ND
 Dispersion Device/Period: Manual/2 hr
 Type & Amount of Agent: Defloc. & 1.0
 Laboratory Comments:

The Material Was Was Not
 The Material Tested Met Did Not Meet Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District



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Particle-Size Analysis of Soil

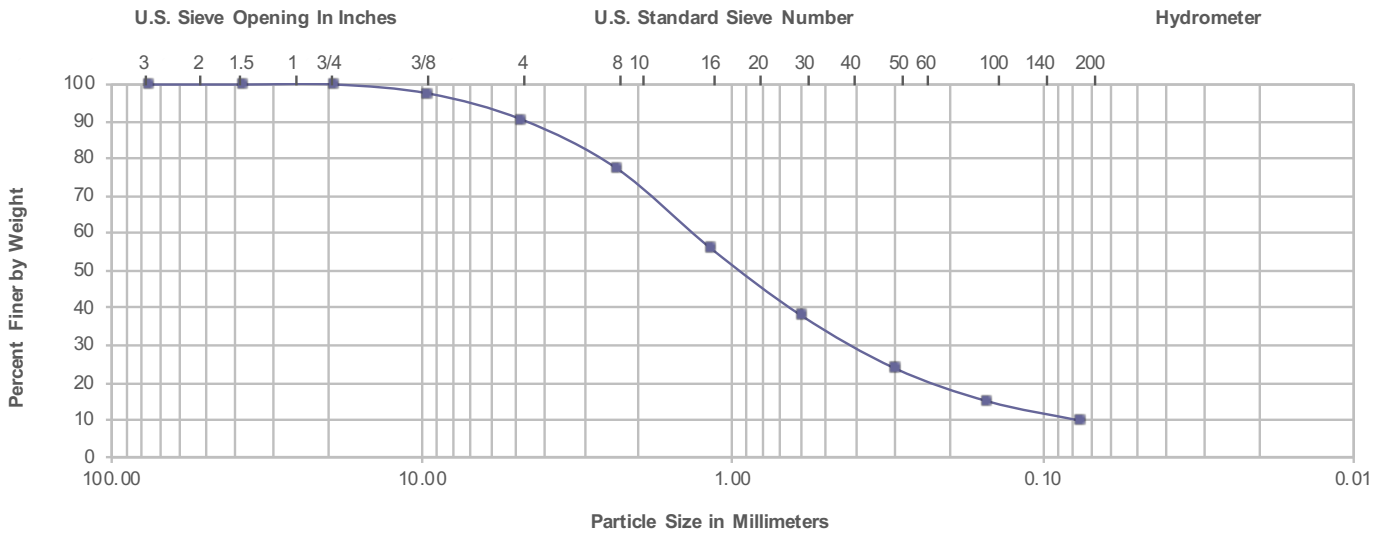
D422, D1140, D2487

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252321 Gravel (%): 9.5% Sand (%): 80.8% Fines (%): 9.7%

Classification, ASTM D2487: (SPSM) Poorly graded sand with silt
 Sample Origin: Boring Six at 5' to 10'
 Laboratory Remarks:



C _u	C _c	Moisture	D ₁₀₀	D ₆₀	D ₃₀	D ₁₀	LL	PL	PI	SG	FM	SE
NA	NA	2.3%	15.109	1.394	0.431	0.000	ND	ND	ND	ND	ND	ND

Method / Procedure Used: D422, D1140
 Size of Initial Dry Mass (g): 528.3
 Determination of Dry Mass: D2216
 Particles; Shape, Hardness: ND
 Dispersion Device/Period: Manual/2 hr
 Type & Amount of Agent: Defloc. & 1.0
 Laboratory Comments:

The Material Was Was Not
 The Material Tested Met Did Not Meet Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District



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Particle-Size Analysis of Soil

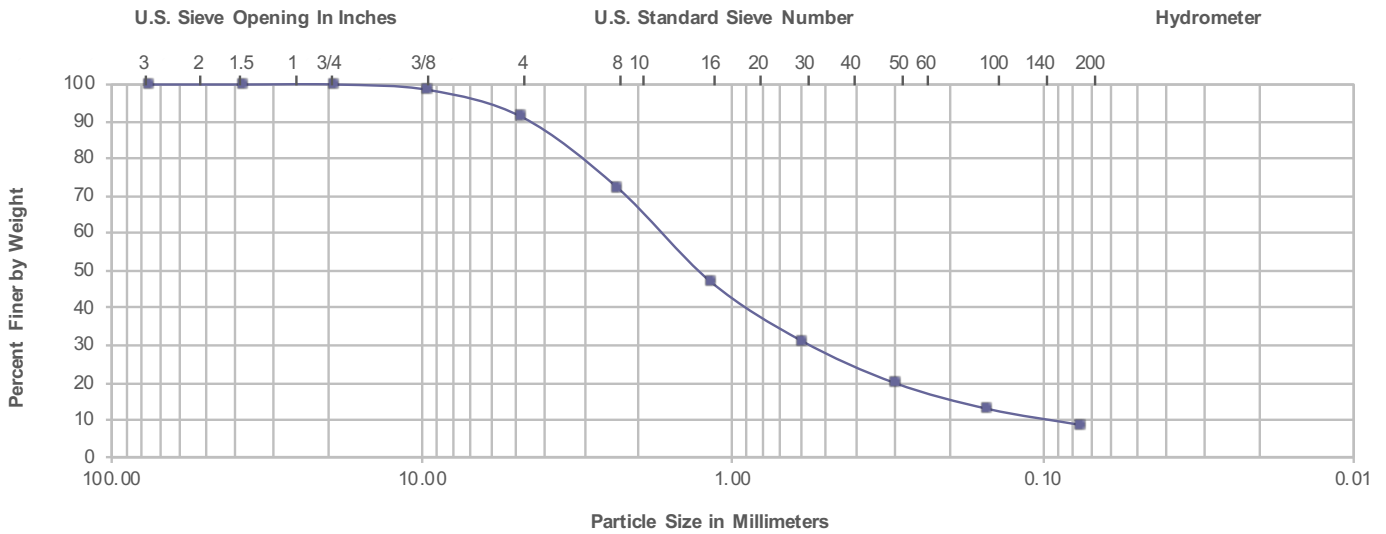
D422, D1140, D2487

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252322 Gravel (%): 8.8% Sand (%): 82.6% Fines (%): 8.6%

Classification, ASTM D2487: (SPSM) Poorly graded sand with silt
 Sample Origin: Boring Seven at 5' to 10'
 Laboratory Remarks:



Particle-Size Analysis of Soil

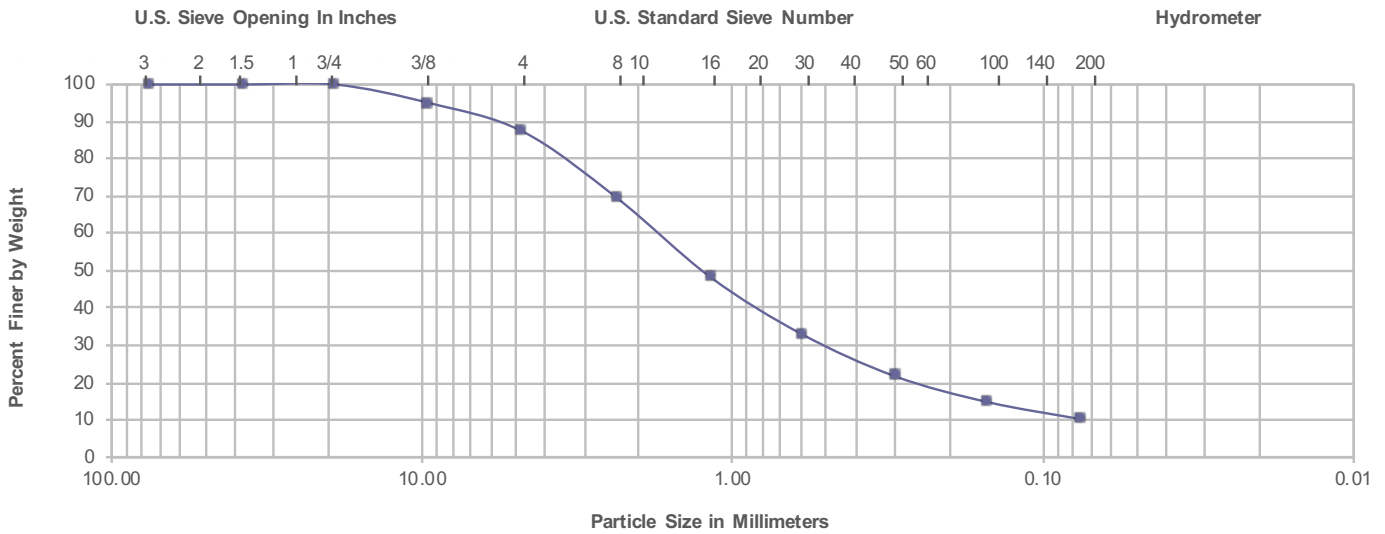
D422, D1140, D2487

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252323 Gravel (%): 12.6% Sand (%): 77.3% Fines (%): 10.1%

Classification, ASTM D2487: (SPSM) Poorly graded sand with silt
 Sample Origin: Boring Eight at 0' to 5'
 Laboratory Remarks:



C _u	C _c	Moisture	D ₁₀₀	D ₆₀	D ₃₀	D ₁₀	LL	PL	PI	SG	FM	SE
NA	NA	1.7%	17.084	1.773	0.520	0.000	ND	ND	ND	ND	ND	ND

Method / Procedure Used: D422, D1140
 Size of Initial Dry Mass (g): 500.1
 Determination of Dry Mass: D2216
 Particles; Shape, Hardness: ND
 Dispersion Device/Period: Manual/2 hr
 Type & Amount of Agent: Defloc. & 1.0
 Laboratory Comments:

The Material Was Met Was Not Met Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District



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Particle-Size Analysis of Soil

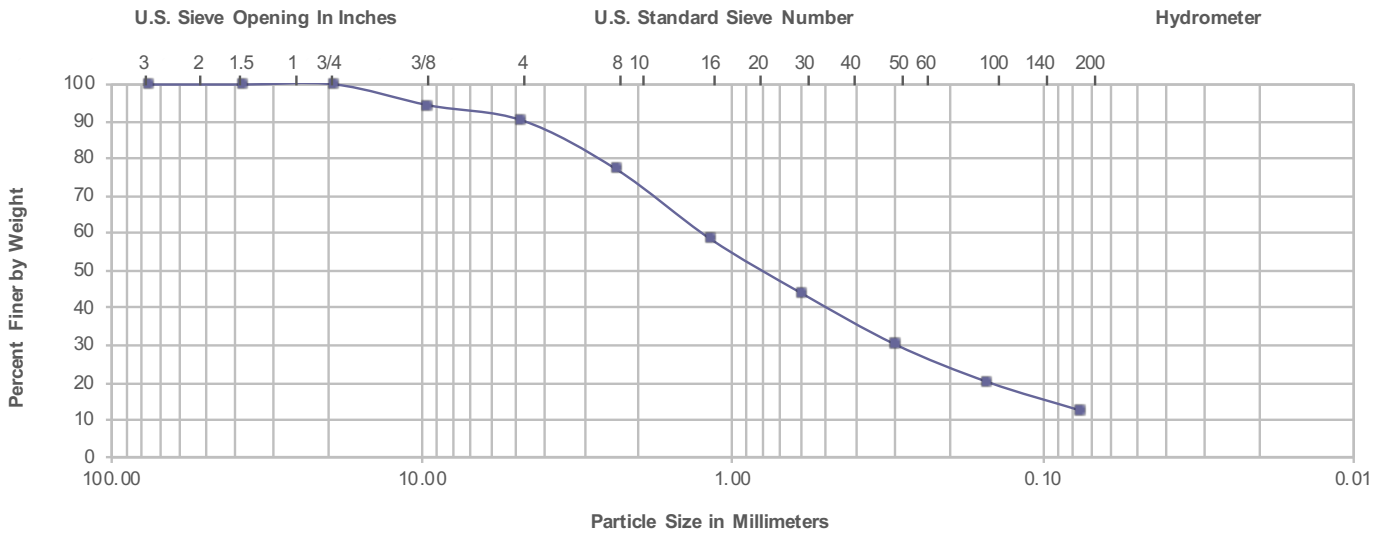
D422, D1140, D2487

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252324 Gravel (%): 9.7% Sand (%): 77.9% Fines (%): 12.4%

Classification, ASTM D2487: (SM) Silty sand
 Sample Origin: Boring Nine at 0' to 5'
 Laboratory Remarks:



C _u	C _c	Moisture	D ₁₀₀	D ₆₀	D ₃₀	D ₁₀	LL	PL	PI	SG	FM	SE
NA	NA	2.3%	17.303	1.272	0.296	0.000	ND	ND	ND	ND	ND	ND

Method / Procedure Used: D422, D1140
 Size of Initial Dry Mass (g): 498.5
 Determination of Dry Mass: D2216
 Particles; Shape, Hardness: ND
 Dispersion Device/Period: Manual/2 hr
 Type & Amount of Agent: Defloc. & 1.0
 Laboratory Comments:

The Material Was Was Not
 The Material Tested Met Did Not Meet Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District



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Particle-Size Analysis of Soil

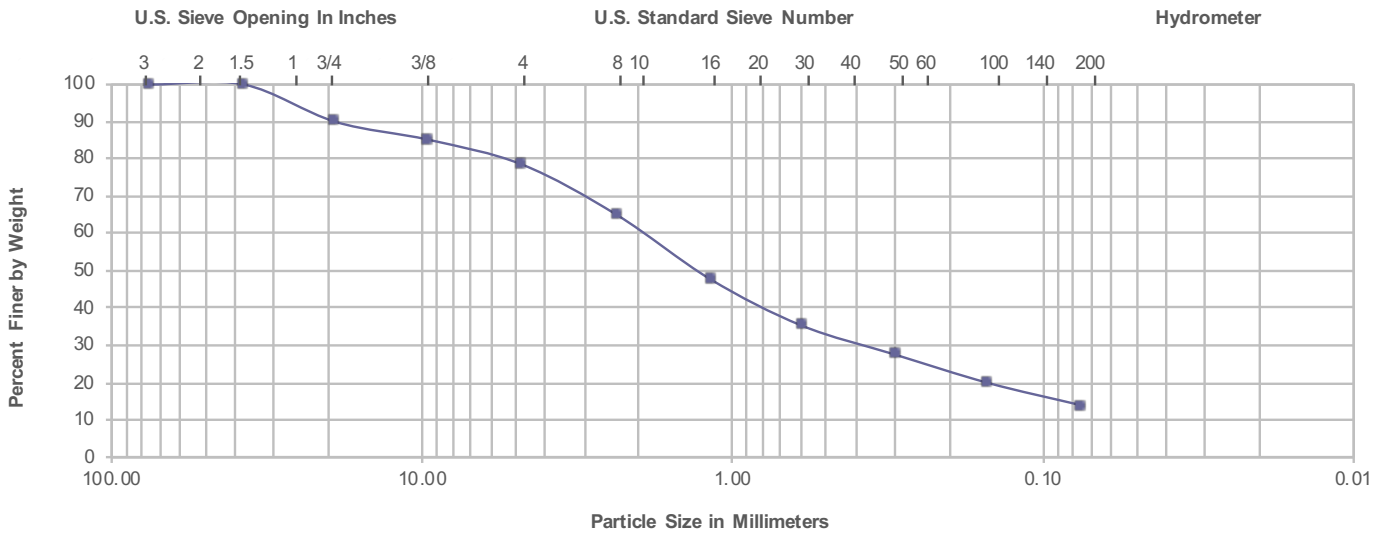
D422, D1140, D2487

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252325 Gravel (%): 21.5% Sand (%): 64.6% Fines (%): 13.9%

Classification, ASTM D2487: (SM) Silty sand with gravel
 Sample Origin: Boring Ten at 0' to 5'
 Laboratory Remarks:



C _u	C _c	Moisture	D ₁₀₀	D ₆₀	D ₃₀	D ₁₀	LL	PL	PI	SG	FM	SE
NA	NA	6.5%	35.655	1.949	0.395	0.000	ND	ND	ND	ND	ND	ND

Method / Procedure Used: D422, D1140
 Size of Initial Dry Mass (g): 507.8
 Determination of Dry Mass: D2216
 Particles; Shape, Hardness: ND
 Dispersion Device/Period: Manual/2 hr
 Type & Amount of Agent: Defloc. & 1.0
 Laboratory Comments:

The Material Was Met Was Not Met Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District



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Particle-Size Analysis of Soil

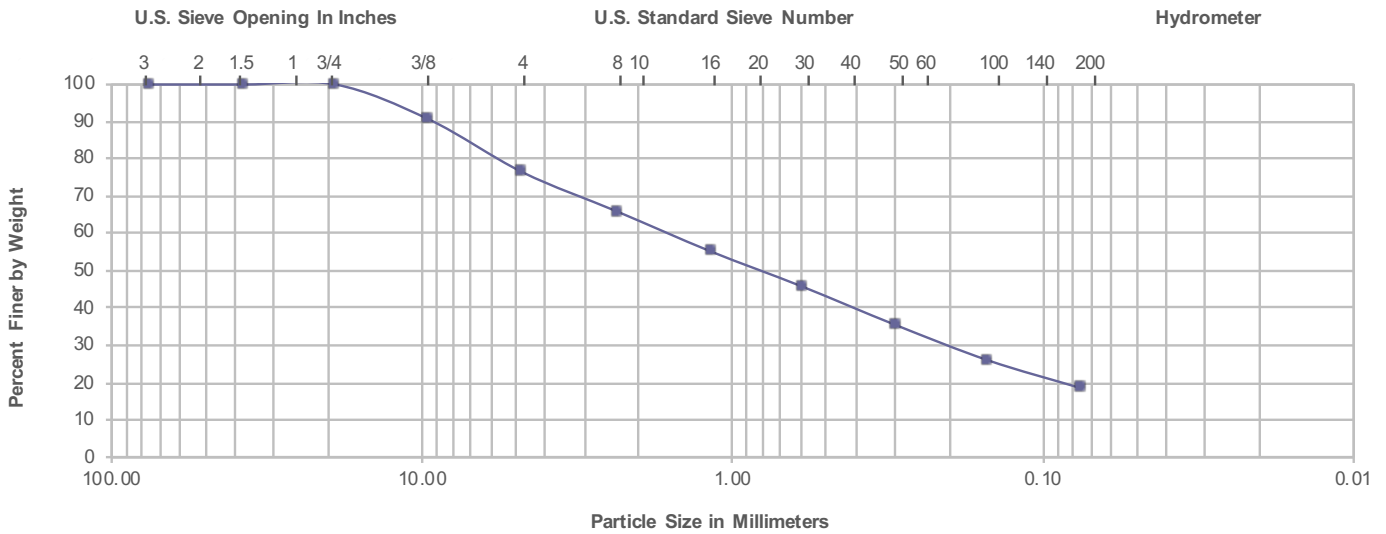
D422, D1140, D2487

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252326 Gravel (%): 23.6% Sand (%): 58.0% Fines (%): 18.4%

Classification, ASTM D2487: (SM) Silty sand with gravel
 Sample Origin: Boring Eleven at 0' to 5'
 Laboratory Remarks:



C _u	C _c	Moisture	D ₁₀₀	D ₆₀	D ₃₀	D ₁₀	LL	PL	PI	SG	FM	SE
NA	NA	5.0%	17.988	1.703	0.214	0.000	ND	ND	ND	ND	ND	ND

Method / Procedure Used: D422, D1140
 Size of Initial Dry Mass (g): 504.9
 Determination of Dry Mass: D2216
 Particles; Shape, Hardness: ND
 Dispersion Device/Period: Manual/2 hr
 Type & Amount of Agent: Defloc. & 1.0
 Laboratory Comments:

The Material Was Met Was Not Met Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District



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Particle-Size Analysis of Soil

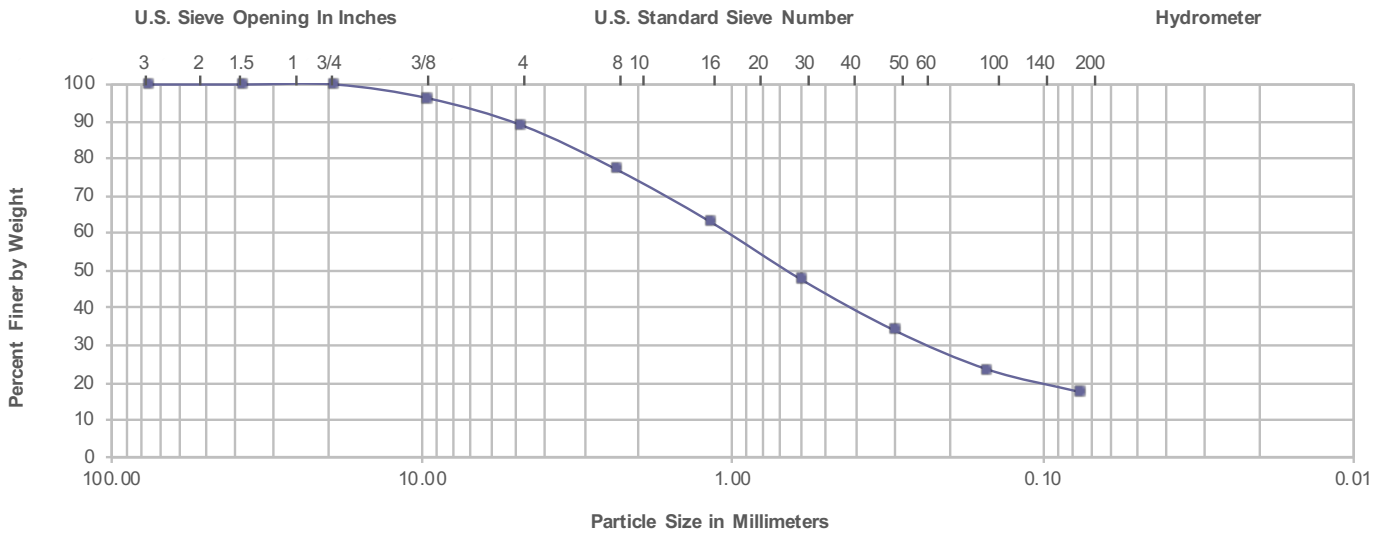
D422, D1140, D2487

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252327 Gravel (%): 11.0% Sand (%): 71.6% Fines (%): 17.4%

Classification, ASTM D2487: (SM) Silty sand
 Sample Origin: Boring Twelve at 0' to 5'
 Laboratory Remarks:



C _u	C _c	Moisture	D ₁₀₀	D ₆₀	D ₃₀	D ₁₀	LL	PL	PI	SG	FM	SE
NA	NA	5.8%	16.471	1.025	0.244	0.000	ND	ND	ND	ND	ND	ND

Method / Procedure Used: D422, D1140
 Size of Initial Dry Mass (g): 505.8
 Determination of Dry Mass: D2216
 Particles; Shape, Hardness: ND
 Dispersion Device/Period: Manual/2 hr
 Type & Amount of Agent: Defloc. & 1.0
 Laboratory Comments:

The Material Was Met Was Not Met Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District



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Particle-Size Analysis of Soil

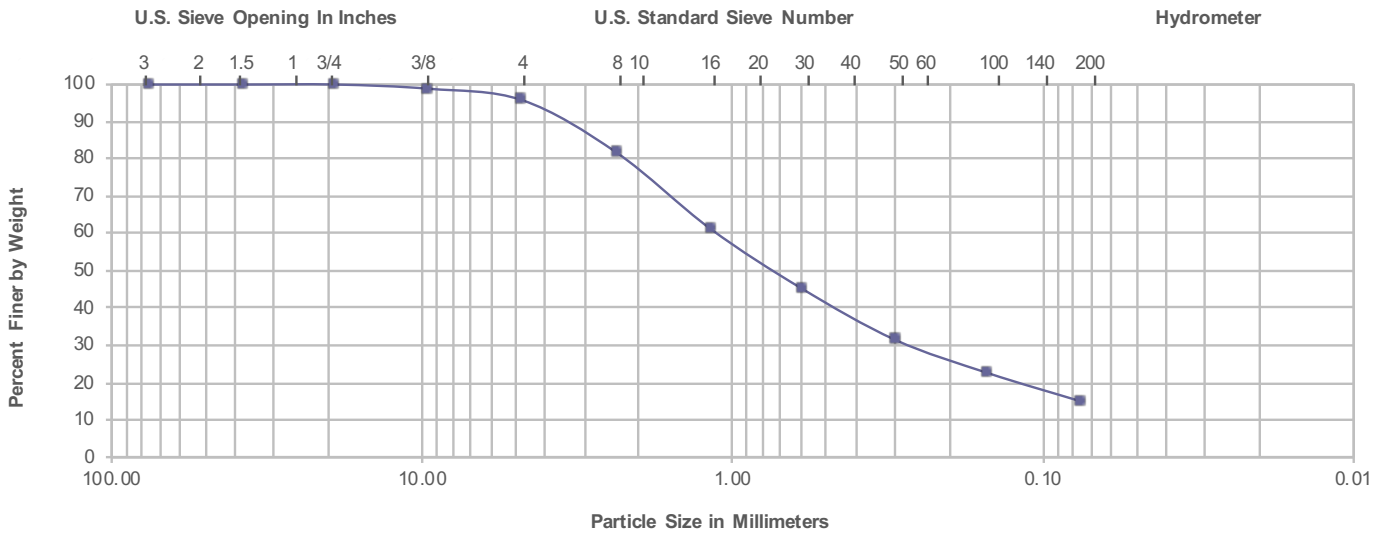
D422, D1140, D2487

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252328 Gravel (%): 4.2% Sand (%): 80.9% Fines (%): 14.9%

Classification, ASTM D2487: (SM) Silty sand
 Sample Origin: Boring Thirteen at 0' to 5'
 Laboratory Remarks:



C _u	C _c	Moisture	D ₁₀₀	D ₆₀	D ₃₀	D ₁₀	LL	PL	PI	SG	FM	SE
NA	NA	2.2%	10.647	1.133	0.275	0.000	ND	ND	ND	ND	ND	ND

Method / Procedure Used: D422, D1140
 Size of Initial Dry Mass (g): 536.3
 Determination of Dry Mass: D2216
 Particles; Shape, Hardness: ND
 Dispersion Device/Period: Manual/2 hr
 Type & Amount of Agent: Defloc. & 1.0
 Laboratory Comments:

The Material Was Met Was Not Met Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District



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Particle-Size Analysis of Soil

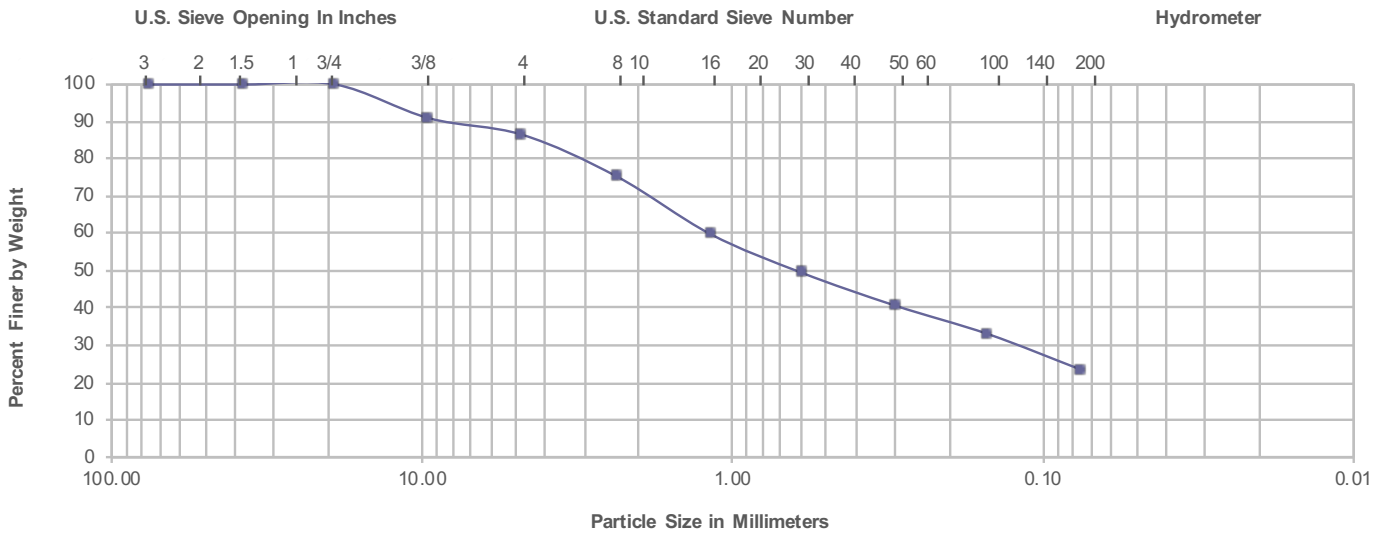
D422, D1140, D2487

Report Date: 1 of 1
 Sheet: C
 Appendix: C
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA11292302 Gravel (%): 13.5% Sand (%): 63.1% Fines (%): 23.4%

Classification, ASTM D2487: (SM) Silty sand
 Sample Origin: Boring Fourteen at 0' to 5'
 Laboratory Remarks: Cardona Road



C _u	C _c	Moisture	D ₁₀₀	D ₆₀	D ₃₀	D ₁₀	LL	PL	PI	SG	FM	SE
NA	NA	1.8%	17.950	1.131	0.119	0.000	ND	ND	ND	ND	ND	ND

Method / Procedure Used: D422, D1140
 Size of Initial Dry Mass (g): 694.0
 Determination of Dry Mass: D2216
 Particles; Shape, Hardness: ND
 Dispersion Device/Period: Manual/2 hr
 Type & Amount of Agent: Defloc. & 1.0
 Laboratory Comments:

The Material Was Met Was Not Met Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District



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Particle-Size Analysis of Soil

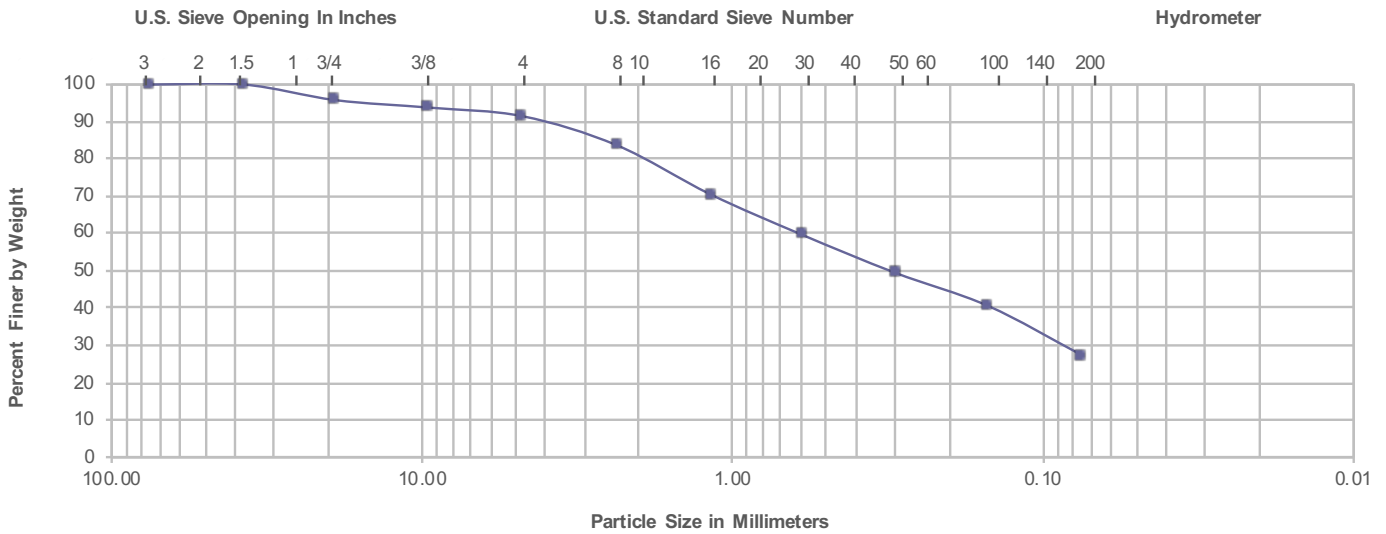
D422, D1140, D2487

Report Date: 1 of 1
 Sheet: C
 Appendix: C
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA11292303 Gravel (%): 8.5% Sand (%): 64.1% Fines (%): 27.4%

Classification, ASTM D2487: (SM) Silty sand
 Sample Origin: Boring Fifteen at 0' to 5'
 Laboratory Remarks: Dachshund Avenue



C _u	C _c	Moisture	D ₁₀₀	D ₆₀	D ₃₀	D ₁₀	LL	PL	PI	SG	FM	SE
NA	NA	1.7%	33.086	0.560	0.084	0.000	ND	ND	ND	ND	ND	ND

Method / Procedure Used: D422, D1140
 Size of Initial Dry Mass (g): 734.9
 Determination of Dry Mass: D2216
 Particles; Shape, Hardness: ND
 Dispersion Device/Period: Manual/2 hr
 Type & Amount of Agent: Defloc. & 1.0
 Laboratory Comments:

The Material Was Was Not Was Not
 The Material Tested Met Did Not Meet Did Not Meet
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District
 Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The requirements of the DSA approved documents.



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Particle-Size Analysis of Soil

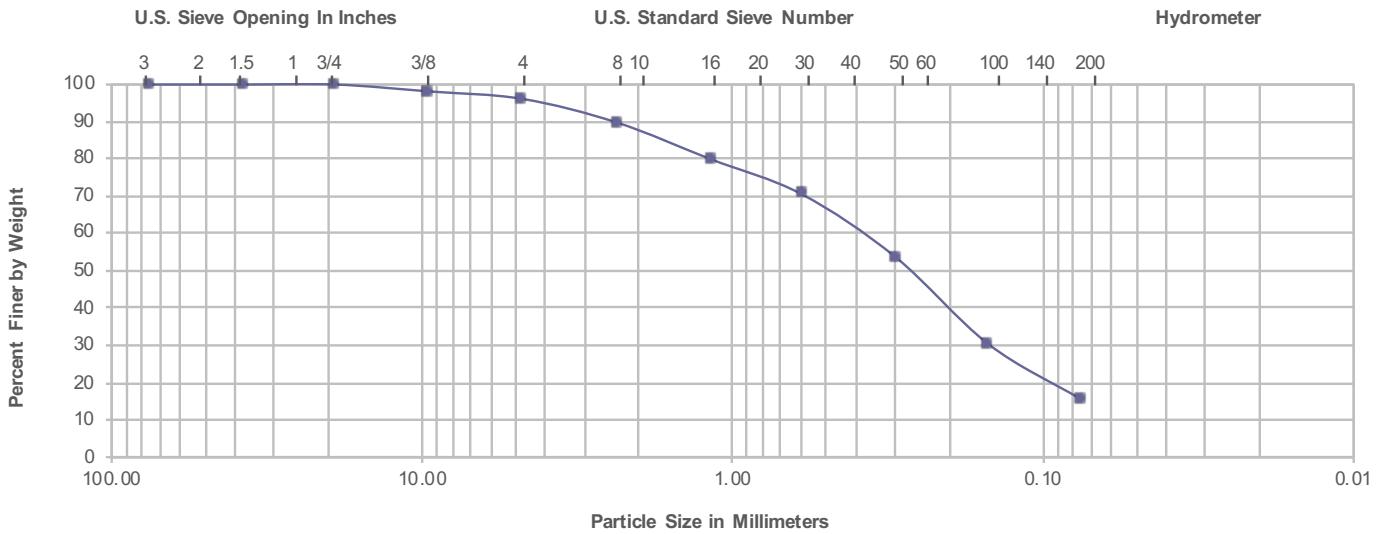
D422, D1140, D2487

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA11292304 Gravel (%): 3.9% Sand (%): 80.7% Fines (%): 15.4%

Classification, ASTM D2487: (SM) Silty sand
 Sample Origin: Boring Sixteen at 0' to 5'
 Laboratory Remarks: Cardona Road



C _u	C _c	Moisture	D ₁₀₀	D ₆₀	D ₃₀	D ₁₀	LL	PL	PI	SG	FM	SE
NA	NA	6.0%	13.850	0.393	0.143	0.000	ND	ND	ND	ND	ND	ND

Method / Procedure Used: D422, D1140
 Size of Initial Dry Mass (g): 688.5
 Determination of Dry Mass: D2216
 Particles; Shape, Hardness: ND
 Dispersion Device/Period: Manual/2 hr
 Type & Amount of Agent: Defloc. & 1.0
 Laboratory Comments:

The Material Was Met Was Not Met Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District



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Particle-Size Analysis of Soil

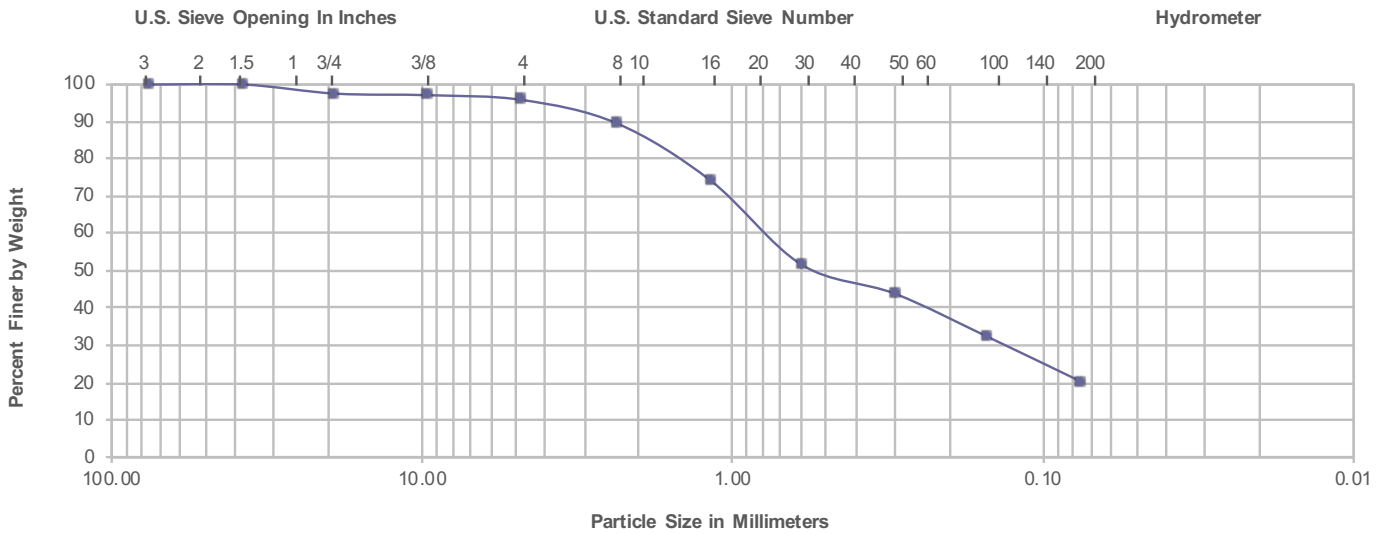
D422, D1140, D2487

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA11292301 Gravel (%): 4.2% Sand (%): 75.7% Fines (%): 20.1%

Classification, ASTM D2487: (SM) Silty sand
 Sample Origin: Boring Seventeen at 0' to 5'
 Laboratory Remarks: Dachsund Avenue



C _u	C _c	Moisture	D ₁₀₀	D ₆₀	D ₃₀	D ₁₀	LL	PL	PI	SG	FM	SE
NA	NA	1.2%	29.922	0.788	0.130	0.000	ND	ND	ND	ND	ND	ND

Method / Procedure Used: D422, D1140
 Size of Initial Dry Mass (g): 1015.9
 Determination of Dry Mass: D2216
 Particles; Shape, Hardness: ND
 Dispersion Device/Period: Manual/2 hr
 Type & Amount of Agent: Defloc. & 1.0
 Laboratory Comments:

The Material Was Met Was Not Met Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District



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Laboratory Compaction Characteristics

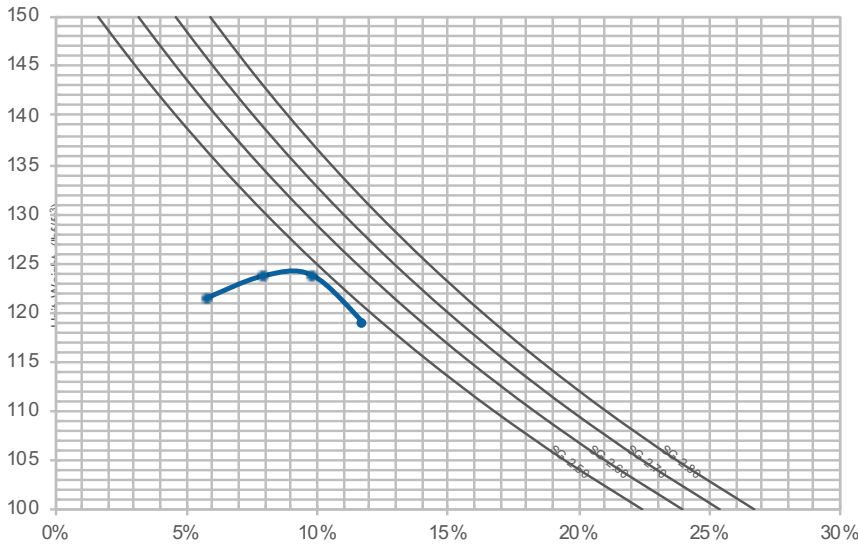
ASTM D1557, D2488

Report Date: 12/01/23
 Sheet: 1 of 1
 Attachment:
 Permit No.: C
 Client Project No.:
 Other:
 DSA File No.:
 DSA Application No.:
 DSA LEA No.:

Project Number: 3813.006.600
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252301 Maximum Dry Unit Weight (lb/ft³): 124.1 Optimum Moisture Content (%): 9.2

Classification, ASTM D2488: (SM) Silty sand
 Sample Origin: Boring One at 0' to 5'
 Laboratory Remarks:



Tested By: DRS
 Received Moisture: 3.8%
 Preparation: Wet
 Specific Gravity:
 SG Method:

Start Weight (lb): 35.0
 Retained on 3/4" (lb): 0.2
 Retained on 3/8" (lb): 0.7
 Retained on No. 4 (lb): 1.9
 Retained on 3/4" (%): 0.6%
 Retained on 3/8" (%): 2.0%
 Retained on No. 4 (%): 5.4%
 Oversize Correction:

Mold Volume Factor: 29.94
 Tare Weight (lb): 4.35
 Rammer Used: Mechanical

Method Used: A B C

Weight of Soil and Tare (lb):	8.64	8.81	8.89	8.79
Wet Weight (g):	349.4	311.7	373.1	347.5
Dry Weight (g):	330.4	288.8	339.8	310.9
Moisture Content (%):	5.8%	7.9%	9.8%	11.8%
Dry Unit Weight (lb/ft ³):	121.5	123.7	123.8	118.9

The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District

Jeremy Beissner
 Reviewed By (Signature)

Jeremy Beissner / Laboratory Manager
 Name / Title



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Laboratory Compaction Characteristics

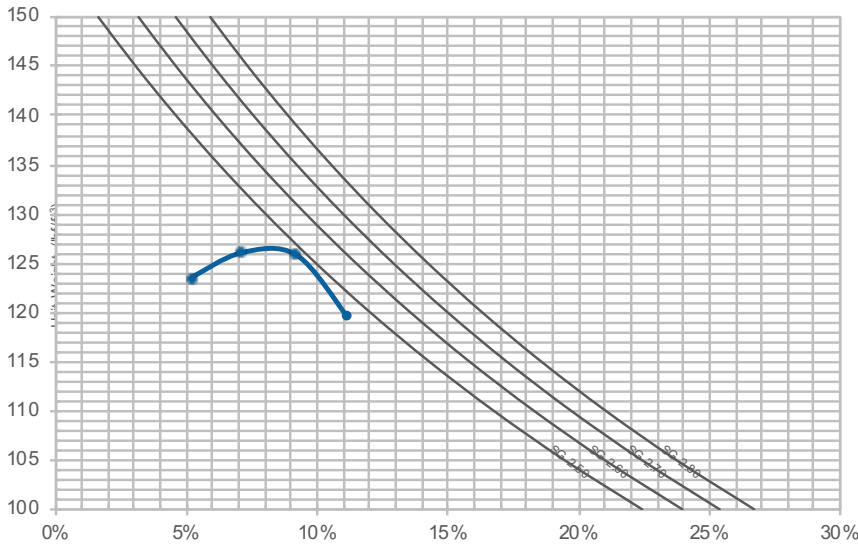
ASTM D1557, D2488

Report Date: 12/01/23
 Sheet: 1 of 1
 Attachment:
 Permit No.: C
 Client Project No.:
 Other:
 DSA File No.:
 DSA Application No.:
 DSA LEA No.:

Project Number: 3813.006.600
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252310 Maximum Dry Unit Weight (lb/ft³): 126.5 Optimum Moisture Content (%): 9.4

Classification, ASTM D2488: (SPSM) Poorly graded sand with silt
 Sample Origin: Boring Two at 5' to 10'
 Laboratory Remarks:



Tested By: DRS
 Received Moisture: 3.6%
 Preparation: Wet
 Specific Gravity:
 SG Method:

Start Weight (lb): 35.0
 Retained on 3/4" (lb): 0.0
 Retained on 3/8" (lb): 0.2
 Retained on No. 4 (lb): 1.4
 Retained on 3/4" (%):
 Retained on 3/8" (%): 0.6%
 Retained on No. 4 (%): 4.0%
 Oversize Correction:

Mold Volume Factor: 29.94
 Tare Weight (lb): 4.35
 Rammer Used: Mechanical

Method Used: A B C

Weight of Soil and Tare (lb):	8.69	8.86	8.94	8.79
Wet Weight (g):	323.0	321.2	317.1	339.4
Dry Weight (g):	307.1	299.9	290.4	305.4
Moisture Content (%):	5.2%	7.1%	9.2%	11.1%
Dry Unit Weight (lb/ft ³):	123.5	126.1	125.9	119.6

The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District

Jeremy Beissner
 Reviewed By (Signature)

Jeremy Beissner / Laboratory Manager
 Name / Title



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Laboratory Compaction Characteristics

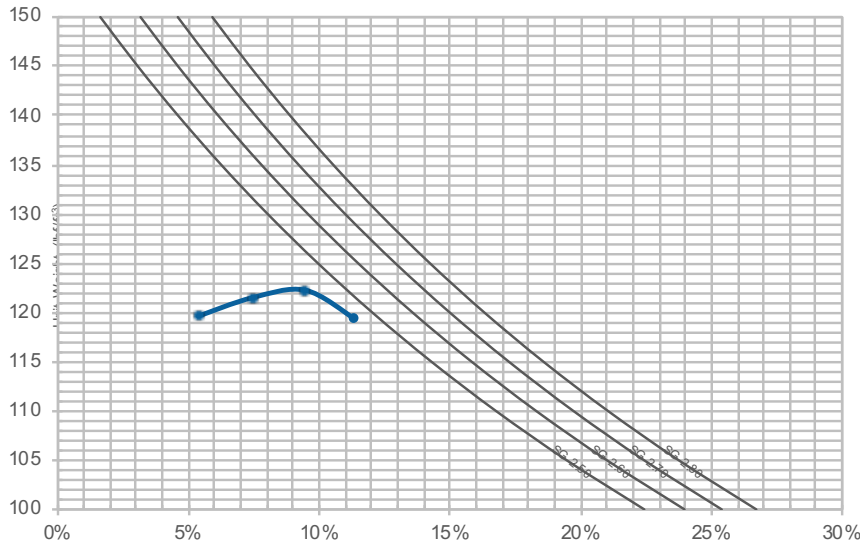
ASTM D1557, D2488

Report Date: 12/01/23
 Sheet: 1 of 1
 Attachment: C
 Permit No.:
 Client Project No.:
 Other:
 DSA File No.:
 DSA Application No.:
 DSA LEA No.:

Project Number: 3813.006.600
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252311 Maximum Dry Unit Weight (lb/ft³): 122.5 Optimum Moisture Content (%): 9.1

Classification, ASTM D2488: (SPSM) Poorly graded sand with silt
 Sample Origin: Boring Three at 0' to 5'
 Laboratory Remarks:



Tested By: DRS
 Received Moisture: 3.4%
 Preparation: Wet
 Specific Gravity:
 SG Method:

Start Weight (lb): 25.0
 Retained on 3/4" (lb): 0.3
 Retained on 3/8" (lb): 1.1
 Retained on No. 4 (lb): 2.6
 Retained on 3/4" (%): 1.2%
 Retained on 3/8" (%): 4.4%
 Retained on No. 4 (%): 10.4%
 Oversize Correction:

Mold Volume Factor: 29.94
 Tare Weight (lb): 4.35
 Rammer Used: Mechanical

Method Used: A B C

Weight of Soil and Tare (lb):	8.56	8.71	8.82	8.79
Wet Weight (g):	349.4	320.8	332.2	321.8
Dry Weight (g):	331.6	298.6	303.5	289.1
Moisture Content (%):	5.4%	7.4%	9.5%	11.3%
Dry Unit Weight (lb/ft ³):	119.6	121.5	122.3	119.4

The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District

Jeremy Beissner

Reviewed By (Signature)

Jeremy Beissner / Laboratory Manager

Name / Title



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Laboratory Compaction Characteristics

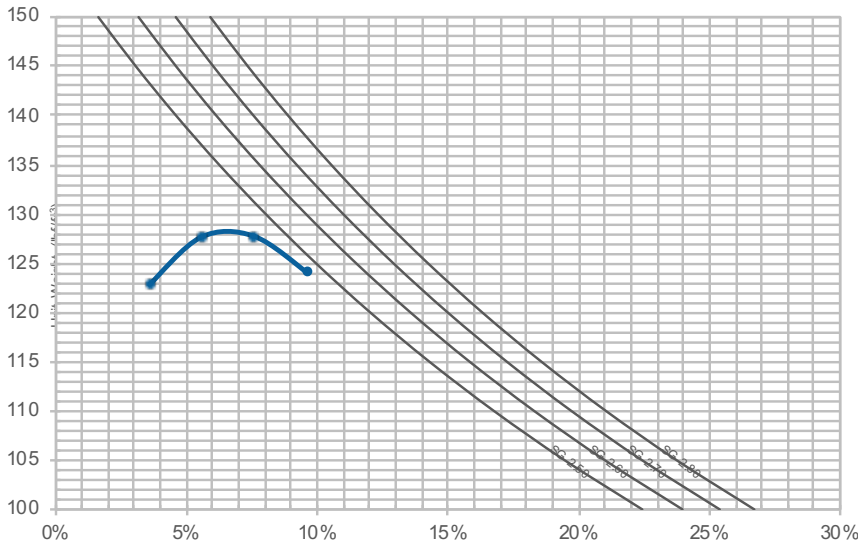
ASTM D1557, D2488

Report Date: 12/01/23
 Sheet: 1 of 1
 Attachment: C
 Permit No.:
 Client Project No.:
 Other:
 DSA File No.:
 DSA Application No.:
 DSA LEA No.:

Project Number: 3813.006.600
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252315 Maximum Dry Unit Weight (lb/ft³): 128.1 Optimum Moisture Content (%): 6.5

Classification, ASTM D2488: (SM) Silty sand
 Sample Origin: Boring Four at 0' to 5'
 Laboratory Remarks:



Tested By: DRS
 Received Moisture: 3.6%
 Preparation: Wet
 Specific Gravity:
 SG Method:

Start Weight (lb): 31.0
 Retained on 3/4" (lb): 0.4
 Retained on 3/8" (lb): 0.8
 Retained on No. 4 (lb): 1.6
 Retained on 3/4" (%): 1.3%
 Retained on 3/8" (%): 2.6%
 Retained on No. 4 (%): 5.2%
 Oversize Correction:

Mold Volume Factor: 29.94
 Tare Weight (lb): 4.35
 Rammer Used: Mechanical

Method Used: A B C

Weight of Soil and Tare (lb):	8.60	8.85	8.94	8.89
Wet Weight (g):	323.7	308.2	318.4	343.2
Dry Weight (g):	312.5	292.0	296.0	313.0
Moisture Content (%):	3.6%	5.5%	7.6%	9.6%
Dry Unit Weight (lb/ft ³):	122.8	127.6	127.8	124.0

The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District

Jeremy Beissner

Reviewed By (Signature)

Jeremy Beissner / Laboratory Manager

Name / Title



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Sand Equivalent of Soils and Fine Aggregate

ASTM D2419

Report Date: 12/01/23
Sheet: 1 of 1
Attachment: C
Permit No.:
Client Project No.:
Other:
DSA File No.:
DSA Application No.:
DSA LEA No.:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Sample ID: JDA10252323 General Compliance Non-Compliance Not Specified

Description: (SPSM) Poorly graded sand with silt
Sample Origin: Boring Eight at 0' to 5'
Laboratory Remarks:

Tested By: JJB
Mechanical/Manual Shaker: Mechanical

Sand Equivalent Value

Amount/Value Allowable

71

-

Amount/Value Allowable Based On:

The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District

Reviewed By (Signature)

Jeremy Beissner/ Laboratory Manager

Name / Title



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Sand Equivalent of Soils and Fine Aggregate

ASTM D2419

Report Date: 12/01/23
Sheet: 1 of 1
Attachment: C
Permit No.:
Client Project No.:
Other:
DSA File No.:
DSA Application No.:
DSA LEA No.:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Sample ID: JDA10252324 General Compliance Non-Compliance Not Specified

Description: (SM) Silty sand with
Sample Origin: Boring Nine at 0' to 5'
Laboratory Remarks:

Tested By: JJB
Mechanical/Manual Shaker: Mechanical

Sand Equivalent Value

Amount/Value Allowable

36

-

Amount/Value Allowable Based On:

The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District

Reviewed By (Signature)

Jeremy Beissner/ Laboratory Manager

Name / Title



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Sand Equivalent of Soils and Fine Aggregate

ASTM D2419

Report Date: 12/01/23
Sheet: 1 of 1
Attachment:
Permit No.: C
Client Project No.:
Other:
DSA File No.:
DSA Application No.:
DSA LEA No.:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Sample ID: JDA10252325 General Compliance Non-Compliance Not Specified

Description: (SM) Silty sand with gravel
Sample Origin: Boring Ten at 0' to 5'
Laboratory Remarks:

Tested By: JJB
Mechanical/Manual Shaker: Mechanical

Sand Equivalent Value

Amount/Value Allowable

24

-

Amount/Value Allowable Based On:

The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District

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Jeremy Beissner/ Laboratory Manager

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Sand Equivalent of Soils and Fine Aggregate

ASTM D2419

Report Date: 12/01/23
Sheet: 1 of 1
Attachment: C
Permit No.:
Client Project No.:
Other:
DSA File No.:
DSA Application No.:
DSA LEA No.:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Sample ID: JDA10252326 General Compliance Non-Compliance Not Specified

Description: (SM) Silty sand with gravel
Sample Origin: Boring Eleven at 0' to 5'
Laboratory Remarks:

Tested By: JJB
Mechanical/Manual Shaker: Mechanical

Sand Equivalent Value

Amount/Value Allowable

24

-

Amount/Value Allowable Based On:

The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District

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concept to completion

ENGINEERING | SURVEYING | TESTING | INSPECTION

Sand Equivalent of Soils and Fine Aggregate

ASTM D2419

Report Date: 12/01/23
Sheet: 1 of 1
Attachment: C
Permit No.:
Client Project No.:
Other:
DSA File No.:
DSA Application No.:
DSA LEA No.:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Sample ID: JDA10252327 General Compliance Non-Compliance Not Specified

Description: (SM) Silty sand
Sample Origin: Boring Twelve at 0' to 5'
Laboratory Remarks:
Tested By: JJB
Mechanical/Manual Shaker: Mechanical

Sand Equivalent Value

Amount/Value Allowable

17

-

Amount/Value Allowable Based On:

The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District

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concept to completion

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Sand Equivalent of Soils and Fine Aggregate

ASTM D2419

Report Date: 12/01/23
Sheet: 1 of 1
Attachment: C
Permit No.:
Client Project No.:
Other:
DSA File No.:
DSA Application No.:
DSA LEA No.:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Sample ID: JDA10252328 General Compliance Non-Compliance Not Specified

Description: (SM) Silty sand with
Sample Origin: Boring Thirteen at 0' to 5'
Laboratory Remarks:

Tested By: JJB
Mechanical/Manual Shaker: Mechanical

Sand Equivalent Value

Amount/Value Allowable

37

-

Amount/Value Allowable Based On:

The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
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cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District

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Consolidation Properties of Soils

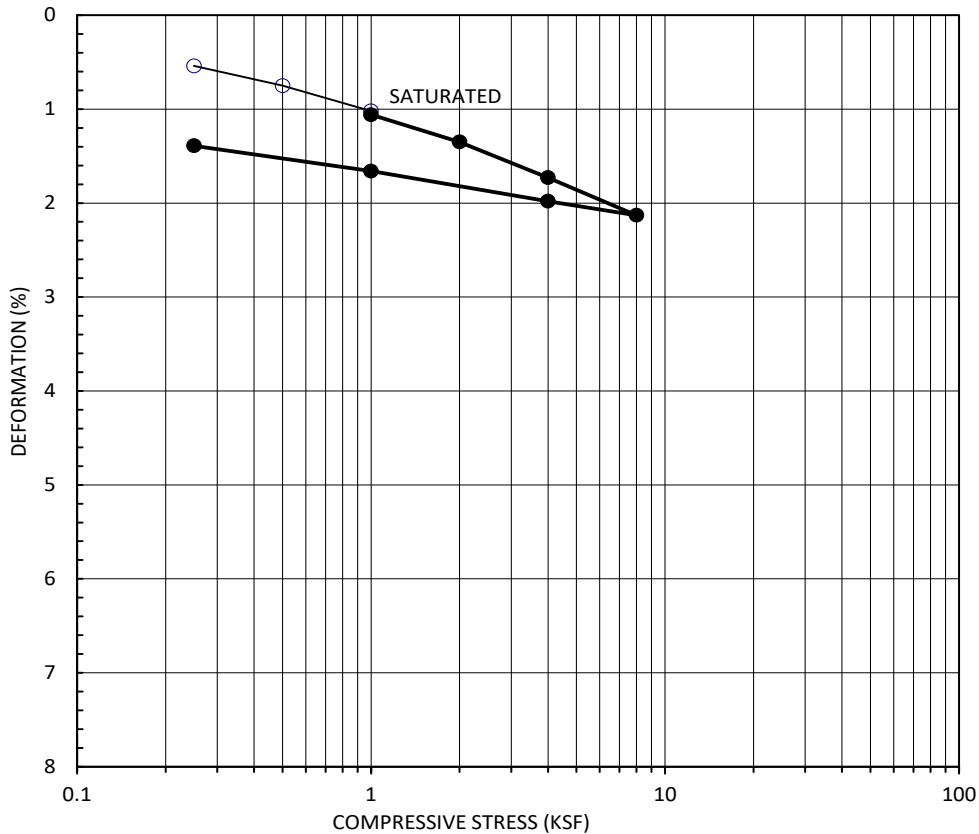
ASTM D2435

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252310 Initial Moisture Content (%): 9.4 Initial Dry Density (pcf): 120.2 Initial Void Ratio: 0.401

Classification, ASTM D2488: (SPSM) Poorly graded sand with silt
 Sample Origin: Boring Two at 5' to 10'
 Laboratory Remarks:



The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District

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R-Value and Expansion Pressure of Compacted Soils

ASTM D2844

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No.:
 Client Project No.:
 Other:
 DSA File No.:
 DSA Application No.:
 DSA LEA No.:

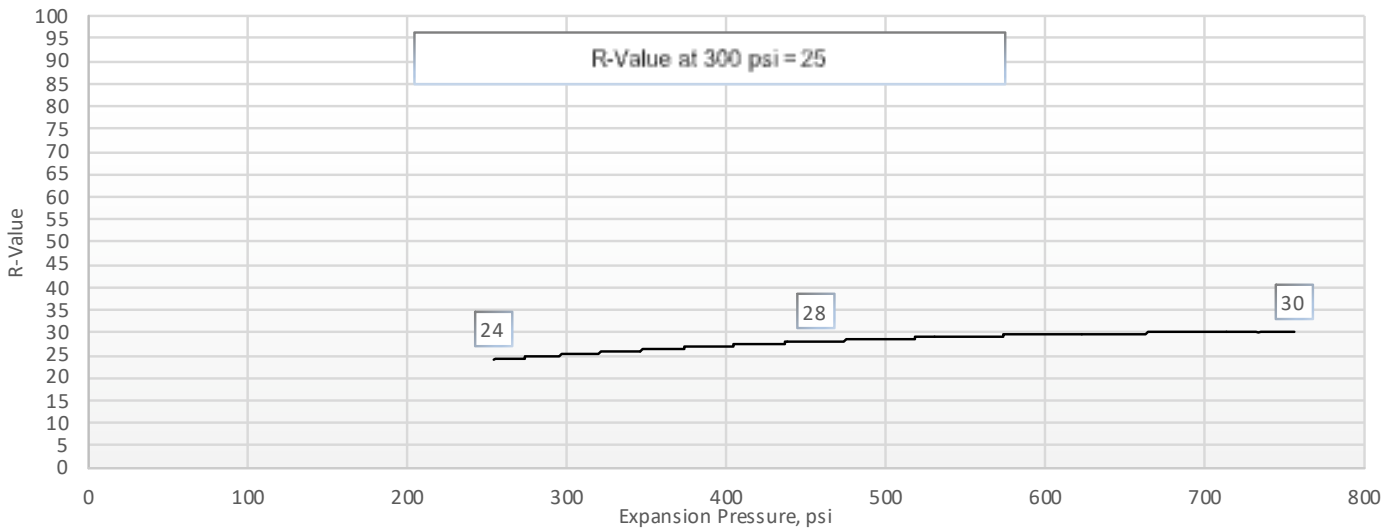
Project Number: 3813.006.600
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252325 General Compliance Non-Compliance Not Specified

Description, D2847: (SM) Silty sand with gravel
 Sample Origin: Boring Ten at 0' to 5', East Side of Warehouse
 Tested By: JJB

Brigette Number:	1	2	3
Moisture Content (%):	12.9	15.1	14.8
Dry Density (pcf):	124.4	118.9	128.6
Exudation Pressure (psi):	756	456	254
Expansion Pressure (psf):	0	0	0
R-Value:	30	28	24

R-Value & Expansion VS. Exudation



The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District

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concept to completion

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R-Value and Expansion Pressure of Compacted Soils

ASTM D2844

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No.:
 Client Project No.:
 Other:
 DSA File No.:
 DSA Application No.:
 DSA LEA No.:

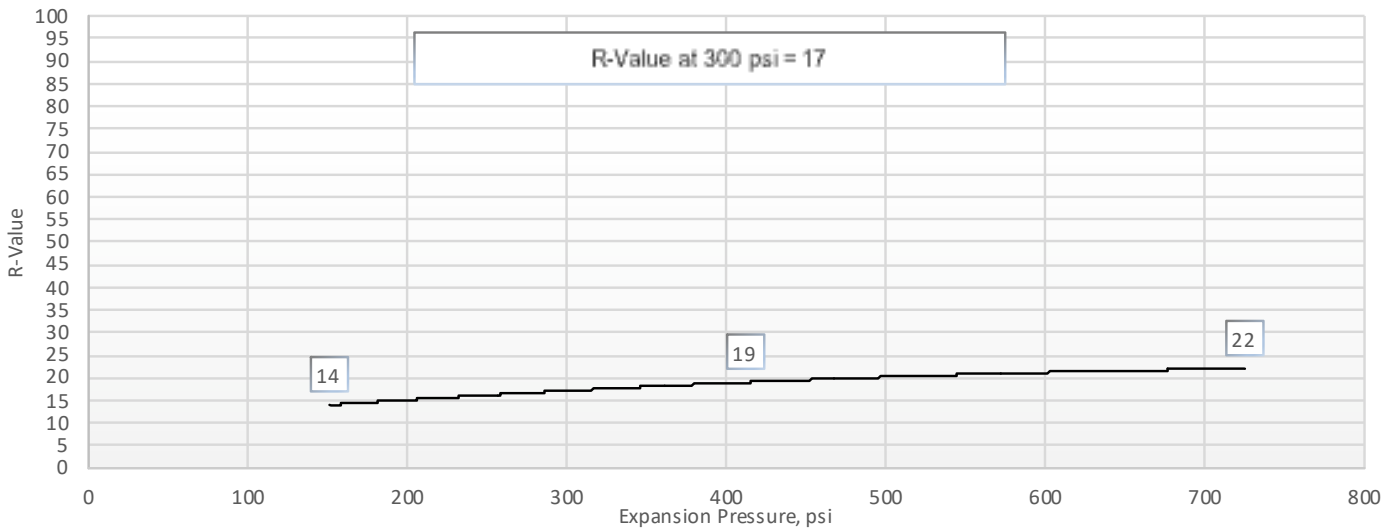
Project Number: 3813.006.600
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252327 General Compliance Non-Compliance Not Specified

Description, D2847: (SM) Silty sand
 Sample Origin: Boring Twelve at 0' to 5', West Side of Warehouse
 Tested By: JJB

Brigette Number:	1	2	3
Moisture Content (%):	10.7	11.8	15.8
Dry Density (pcf):	126.3	125.6	112.4
Exudation Pressure (psi):	725	412	151
Expansion Pressure (psf):	0	0	0
R-Value:	22	19	14

R-Value & Expansion VS. Exudation



The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District

Jeremy Beissner

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Jeremy Beissner / Laboratory Manager

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concept to completion

ENGINEERING | SURVEYING | TESTING | INSPECTION

R-Value and Expansion Pressure of Compacted Soils

ASTM D2844

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No.:
 Client Project No.:
 Other:
 DSA File No.:
 DSA Application No.:
 DSA LEA No.:

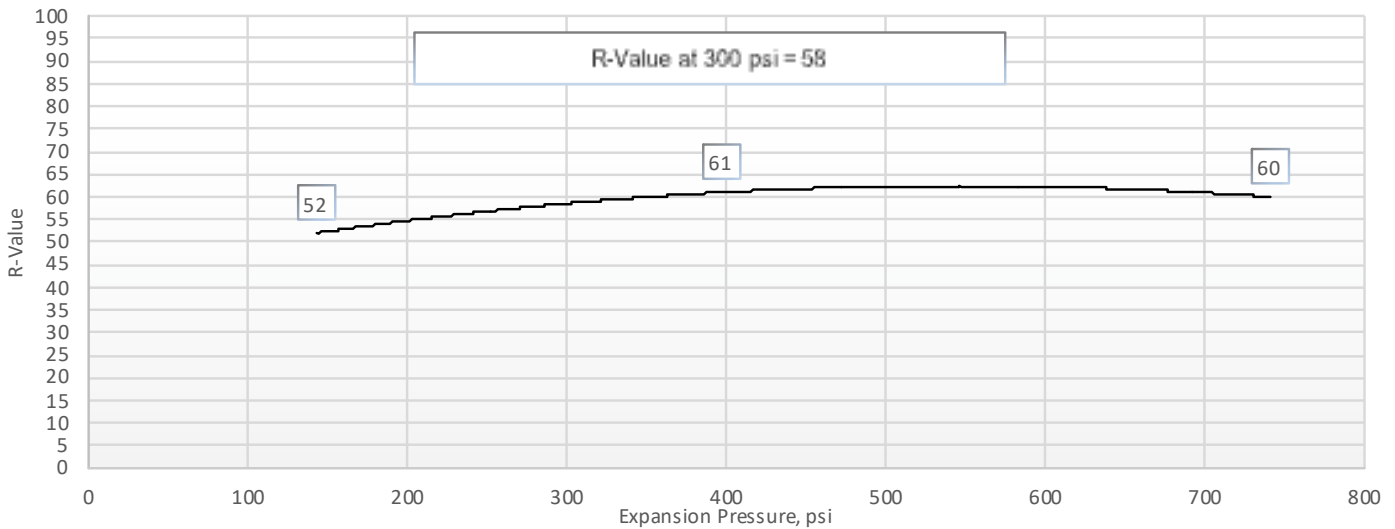
Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA11292302 General Compliance Non-Compliance Not Specified

Description, D2847: (SM) Silty sand
 Sample Origin: Boring Fourteen at 0' to 5', Dachund Avenue
 Tested By: JJB

Brigette Number:	1	2	3
Moisture Content (%):	7.0	7.5	9.1
Dry Density (pcf):	129.1	130.9	127.9
Exudation Pressure (psi):	741	397	143
Expansion Pressure (psf):	0.0135	0.0169	0.0194
R-Value:	60	61	52

R-Value & Expansion VS. Exudation



The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
 The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District

Jeremy Beissner

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Jeremy Beissner / Laboratory Manager

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concept to completion

ENGINEERING | SURVEYING | TESTING | INSPECTION

R-Value and Expansion Pressure of Compacted Soils

ASTM D2844

Report Date:

Sheet: 1 of 1

Appendix: C

Permit No.:

Client Project No.:

Other:

DSA File No.:

DSA Application No.:

DSA LEA No.:

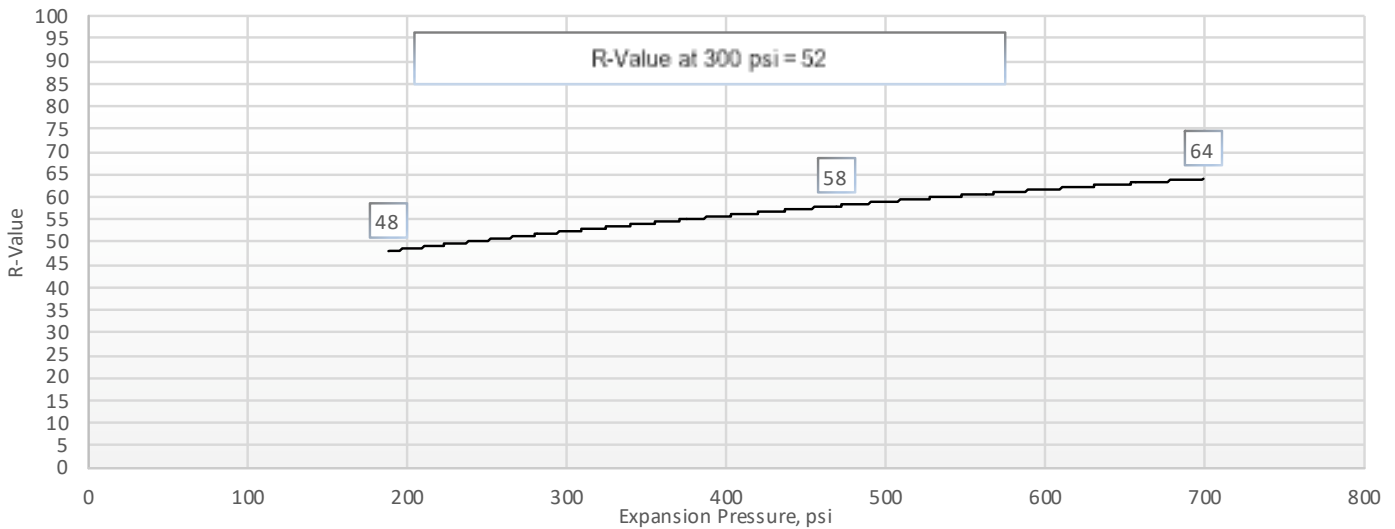
Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA11292303 General Compliance Non-Compliance Not Specified

Description, D2847: (SM) Silty sand
 Sample Origin: Boring Fifteen at 0' to 5' Cardova Avenue
 Tested By: JJB

Brigette Number:	1	2	3
Moisture Content (%):	7.5	8.1	8.8
Dry Density (pcf):	129.9	130.5	127.8
Exudation Pressure (psi):	699	469	188
Expansion Pressure (psf):	0.0031	0.0017	0.0008
R-Value:	64	58	48

R-Value & Expansion VS. Exudation



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 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District

Jeremy Beissner

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Jeremy Beissner / Laboratory Manager

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concept to completion

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Direct Shear Test of Soils

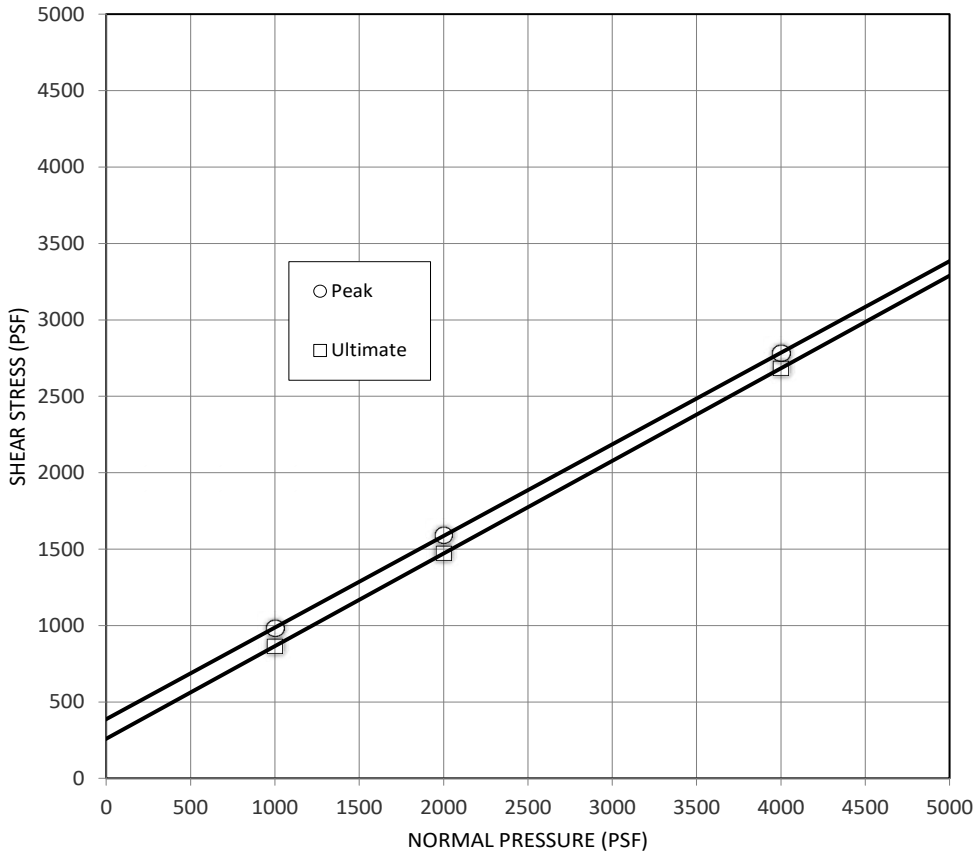
ASTM D3080

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252301 Angle of Internal Friction (°): 31 Peak Cohesion (psf): 388 Ultimate Cohesion (psf): 260

Classification, ASTM D2488: (SM) Silty sand
 Sample Origin: Boring One at 0' to 5'
 Laboratory Remarks: Sample was remolded to 95% relative compaction and optimum moisture.
 Maximum Dry Density: 124.1 pcf Optimum Moisture: 9.2%



The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District

Jeremy Beissner

 Reviewd By (Signature)

Jeremy Beissner/ Laboratory Manager

 Name / Title



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Direct Shear Test of Soils

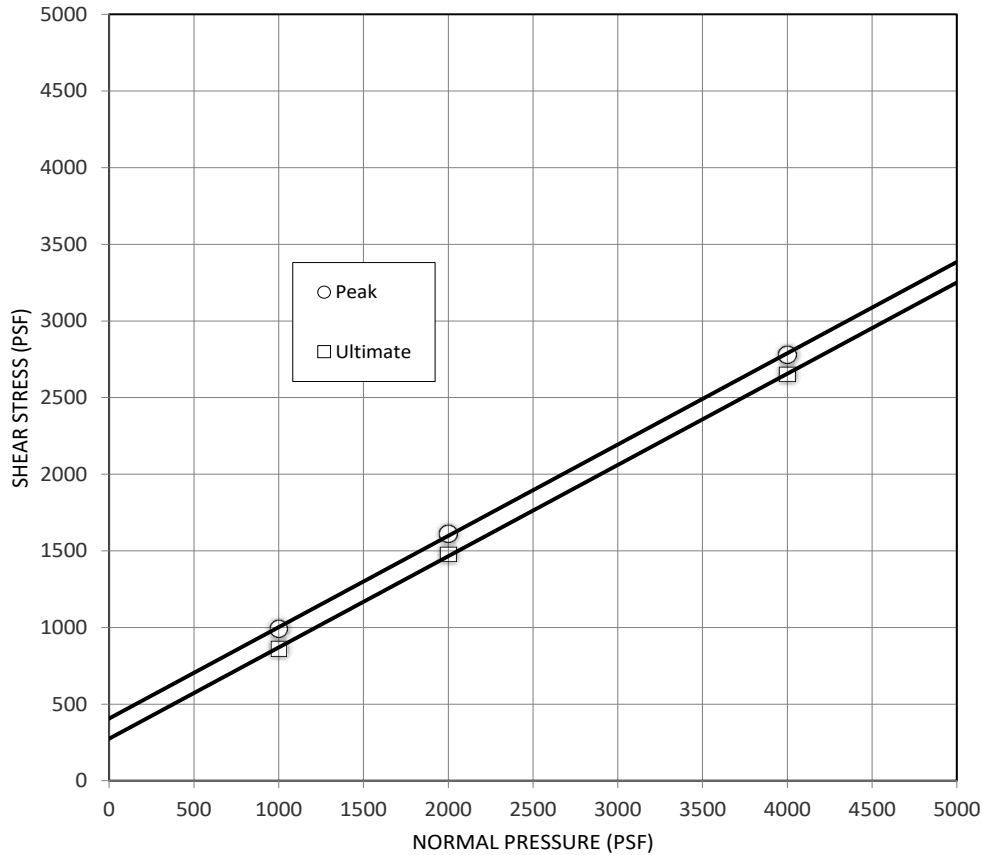
ASTM D3080

Report Date: 12/01/23
 Sheet: 1 of 1
 Appendix: C
 Permit No:
 Client Project No:
 Other:
 DSA File No:
 DSA Application No:
 DSA LEA No:

Project Number: 3813.006.500
 Project Title: Industrial Building
 Project Location: Apple Valley, CA
 Client: Redwood West

Sample ID: JDA10252315 Angle of Internal Friction (°): 31 Peak Cohesion (psf): 406 Ultimate Cohesion (psf): 275

Classification, ASTM D2488: (SM) Silty sand
 Sample Origin: Boring Four at 0' to 5'
 Laboratory Remarks: Sample was remolded to 95% relative compaction and optimum moisture.
 Maximum Dry Density: 128.1 pcf Optimum Moisture: 6.5%



The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
 cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District

Jeremy Beissner

Reviewed By (Signature)

Jeremy Beissner/ Laboratory Manager

Name / Title



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Expansion Index

ASTM D4829

Report Date: 12/01/23
Sheet: 1 of 1
Attachment: C
Permit No.:
Client Project No.:
Other:
DSA File No.:
DSA Application No.:
DSA LEA No.:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Sample ID: JDA10252311 General Compliance Non-Compliance Not Specified

Classification, ASTM D2487: (SPSM) Poorly graded sand with silt
Sample Origin: Boring Three at 0' to 5'
Laboratory Remarks:

Tested By: JJB
Method/Procedure: ASTM D4829

Expansion Index

Value: 2

Expansion Index

Potential Expansion

0 - 20

Very Low

21 - 50

Low

51 - 90

Medium

91 - 130

High

> 130

Very High

The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
The Material Tested Met Did Not Meet The requirements of the DSA approved documents.
cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District



Reviewed By (Signature)

Jeremy Beissner / Laboratory Manager

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Corrosion Potential

CT 643, 422, 417, 643

Report Date: 12/01/23
Sheet: 1 of 1
Appendix: C
Permit No:
Client Project No:
Other:
DSA File No:
DSA Application No:
DSA LEA No:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Sample ID: JDA10252310

Classification, ASTM D2487: (SPSM) Poorly graded sand with silt
Sample Origin: Boring Two at 5' to 10'
Laboratory Remarks:

Analysis	Result	Units	Test Method
Minimum Resistivity	1,500	ohm-cm	CT 643
Chloride Content	260	ppm	CT 422
Sulfate Content	0.006	%	CT 417
pH	8.85	pH units	CT 643

The Material Was Was Not Was Not
The Material Tested Met Did Not Meet Did Not Meet
Sampled & tested in accordance with the reqs. of the DSA approved documents.
The requirements of the DSA approved documents.
cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District



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Corrosion Potential

CT 643, 422, 417, 643

Report Date: 12/01/23
Sheet: 1 of 1
Appendix: C
Permit No:
Client Project No:
Other:
DSA File No:
DSA Application No:
DSA LEA No:

Project Number: 3813.006.500
Project Title: Industrial Building
Project Location: Apple Valley, CA
Client: Redwood West

Sample ID: JDA10252311

Classification, ASTM D2487: (SPSM) Poorly graded sand with silt
Sample Origin: Boring Three at 0' to 5'
Laboratory Remarks:

Analysis	Result	Units	Test Method
Minimum Resistivity	260	ohm-cm	CT 643
Chloride Content	1230	ppm	CT 422
Sulfate Content	0.177	%	CT 417
pH	7.86	pH units	CT 643

The Material Was Was Not Sampled & tested in accordance with the reqs. of the DSA approved documents.
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cc: Project Architect, Structural Engineer, Project Inspector, DSA Regional Office, School District



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Jeremy Beissner/ Laboratory Manager

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760-256-2068 | www.merrelljohnson.com